## Appendix F

## Model Development and Application

F1. Model Development (Halcrow)
F2. Model Applications (Halcrow)
F3. 2021 and 2031 Corridor Deficiencies

## F1. Model Development (Halcrow)

F1.1 November 2010 Technical Memorandum
F1. 2 July 6, 2010 Memorandum

## F1.1 November 2010 Technical Memorandum

# City of Vaughan 

Transportation Master Plan Study
Vaughan Sub-Area Model (VSAM)
Technical Report
November 2010


Halcrow Consulting Inc

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November 2010

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## City of Vaughan

## Transportation Master Plan Study <br> Vaughan Sub-Area Model (VSAM) <br> Technical Report

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## 1 Introduction

### 1.1 Background

As part of the on-going Vaughan Transportation Master Plan (TMP) Study, Halcrow was commissioned by the City of Vaughan and the prime consultant, AECOM, to develop a sub-area travel demand model for the City of Vaughan based on a local refinement of the York Rapid Transit Plan (YRTP) regional model. For the purposes of this TMP Study, Halcrow refined, updated and validated the YRTP model to a 2006 base to reflect the more current travel habits and traffic count information specific to the study area. This new Vaughan Sub-Area Model (VSAM) will be used to evaluate future transportation infrastructure requirements for the City of Vaughan in the long and short terms (2011, 2021 and 2031), in support of the study objectives to encourage public transit use, alleviate congestion and promote liveable street networks and neighbourhoods within the City. The model will also be used by the City Staff for their in-house traffic forecasting after completion of the TMP Study. This technical report documents the VSAM development and validation process as well as the resulting forecasts generated for this study.

### 1.2 Key Tasks

Some of the key tasks involved in the VSAM model development and validation are described below:

- Refined traffic zone system within the City of Vaughan to provide more realistic representation of the network and of walking distances to transit stations/stops;
- Updated the existing YRTP model base year from 2001 to 2006 on the latest EMME 3 platform;
- Added local collector roads which are under the jurisdiction of the City of Vaughan for local traffic diversion and infiltration analysis;
- Improved the YRTP modelling process to provide more reliable auto traffic and transit ridership forecast;
- Validated the model results at the screenline level for the base year 2006;
- Developed a PM model based on the calibrated AM model to estimate future PM peak hour auto traffic demand; and
- Developed future 2031 base model using updated land use and demographic data, road networks and external matrices.


### 1.3 Deliverables

The following deliverables were prepared for this VSAM study:

- A refined sub-area model for the base year 2006 and 2031;
- 2006 and 2031 auto and transit base networks;
- An enhanced model input preparation spreadsheet to incorporate land use and other necessary zonal data; and
- A set of EMME macros for implementing base and future year model runs.


### 1.4 Report Organization

This report is organized into six sections. Section 2 describes the model structure, model updates and model inputs for the 2006 base year. Section 3 presents the validation results for the AM model. Section 4 describes the PM modelling approach, shopping trip analysis and the PM model validation. Section 5 summarizes the 2031 model assumptions and the resulting forecasts. Finally, Section 6 provides a summary and conclusions.

## 2 AM Model Development

### 2.1 Introduction

This chapter documents the model development and validation of the AM Vaughan SubAreal Model (VSAM). This chapter is organized into five sections:

- Section 2.2 describes the YRTP and Western Vaughan IEA model
- Section 2.3 provides an overview of the VSAM model
- Section 2.4 documents the model update process
- Section 2.5 describes the traffic zone system and demographic inputs
- Section 2.6 describes the road and transit networks


### 2.2 YRTP and Western Vaughan IEA Models

VSAM is an updated version of the YRTP regional model, which is a traditional 4-Stage EMME/2-based model developed by IBI Group for the York Region Rapid Transit Plan (YRTP) Study in 2003. The YRTP model was designed primarily for forecasting transit ridership on the proposed rapid transit lines along the Yonge Street and Highway 7 rapid transit corridors at the time of the study. It has the capabilities to model three trip purposes including work, school and other trips during the AM peak period (6AM-9AM) by the motorized travel modes.

The YRTP model uses the standard 2001 GTA traffic zone system developed by the Data Management Group, with enhanced zonal detail in the transit-oriented intensification areas to better reflect variations in walk access to transit stops. This multi-modal model was originally calibrated using 2001 travel data collected as part of the 2001 Transportation Tomorrow Survey (TTS) and 2001 traffic and transit count information. It estimates peak period traffic demands for the base and future horizon years at the arterial and collector road level within the Greater Toronto and Hamilton Area (GTHA) as well as transit demands for planned rapid transit facilities and feeder bus routes.

In the fall of 2007, Halcrow was commissioned by York Region and UMA (now AECOM) to update the YRTP model and provide travel demand forecasts for different auto and transit network alternatives in support of the Western Vaughan Individual Environmental Assessment (IEA) Study. While the traffic zone system of the YRTP model was retained, demographics and networks were updated to 2006 base for the Western Vaughan IEA. The resulting 2006 trip matrices were validated against the 2006 TTS and traffic assignment were checked against 2006 traffic counts. Given the full 4 -stage multi-modal
modelling capability and the geographic similarity of the revised Western Vaughan IEA model, it was the logical choice as the basis for the VSAM.

### 2.3 VSAM Overview

2.3.1 Modelling Approach

VSAM is a standard travel demand model that estimates the overall trip-making decisions of individuals within the GTHA. Such models are often referred to as "macroscopic" models. The techniques used and degree of detail of model results are in sharp contrast to traffic simulation or "microscopic" models that simulate the expected behaviour of individual trip makers as they negotiate their respective paths through a section of the transportation network.

The principal inputs to VSAM are:

- Land use (e.g. population and employment information by traffic zone)
- Road network (e.g. number of lanes, lane capacity, etc.)
- Transit network and services (e.g. route alignment, stops, headways, etc.)
- Economic and travel cost data (e.g., value of time, vehicle operating costs, parking costs, transit fares, etc.)

The model outputs are forecasts of travel volumes and times and out-of-pocket costs by travel mode for:

- All origin-destination pairs
- Each roadway link in the coded road network
- Each transit line segment in the coded transit network

VSAM is a multi-modal transportation model that estimates travel demand in the following steps:

- Trip generation
- Trip distribution
- Mode split
- Trip assignment

These steps are covered in the following sections in more detail.

Trip Generation
Trip generation equations estimate the number of trips produced and attracted by each traffic zone during the AM peak period based on demographic data, trip rates and
calibrated regression equations for the following major trip purposes and the corresponding sub-categories:

- Work trips for 3 occupational groups (Office / Manufacturing / Professional)
- $\quad$ School trips (Secondary / Post-secondary)
- Other trips

External trips with an origin or destination outside the GTHA are not modelled specifically in VSAM as they were extracted directly from the 2006 TTS data with a standard growth rate assumed for future years. These external trips are added to the internal trips after the mode split stage and prior to the trip assignment procedure.
2.3.4 Mode Split

A multinomial logit sub-model is calibrated to estimate the percentage of work trips by the following motorized modes:

- Auto driver
- Public transit with walk access
- Public transit with auto access (Park/Kiss-and-ride at subway stations)
- GO Rail with walk or public transit access
- GO Rail with auto access (Park/Kiss-and-ride at GO Rail stations)

The following input variables are used in the multinomial logit formulation:

- Level-of-service and cost (e.g. In-vehicle travel time, auto operating cost, parking cost etc.)
- Transit supportive land use variables (e.g. Urban density, land use mix etc.)
- Percentage of households without auto access estimated by an autoownership sub-model

A standard logit function is used for estimating modal share for post-secondary school trips while observed modal split rates for each planning district are used for secondary school and other trips.

### 2.3.5 Trip Assignment

Trip assignment is the final step of the 4-stage modelling process. A standard generalized cost equilibrium auto assignment is implemented in this stage to model route choices for the auto drivers as well as transit patronage based on the weighted generalized cost (i.e. travel time and cost) between each O-D pair. These travel times for auto and transit were estimated using volume-delay functions and transit time functions that are described in Section 2.6 of this chapter. Park-and-ride trips are also assigned onto the auto subnetwork in this stage to allow potential users to access all the commuter parking lots for transit stations within the GTHA (e.g. TTC Finch Station). Peak hour auto traffic are estimated for assigned traffic by applying appropriate peak hour factors (PHFs) that reflect observed peaking characteristics for the auto mode.

The YRTP model does not model high-occupancy vehicles (HOV). VSAM represents HOV lanes by assuming two thirds of the capacities of the general purpose lanes in order to reflect lower traffic usage in the trips assignment stage. Trucks are not modelled in VSAM. Observed truck percentages on specific roadways or classes of roadway can be applied to the assigned auto volumes manually (post-model run) to estimate the approximate impact of truck volumes on traffic conditions.
2.3.6 Modelling Process

The modelling process is iterative, involving the recycling of outputs to achieve "convergence". For example, travel times or travel costs are the key inputs for the trip distribution process. Whenever significant changes occur that affect travel times or travel costs in the trip distribution stage, the subsequent modelling procedures (i.e. mode split and trip assignment) are also affected. Therefore, the model must be cycled multiple times until input travel times in the trip distribution stage and the resulting output times generated from the trip assignment stage are consistent in order to achieve model convergence. It has been tested that four cycles are sufficient for the VSAM to meet the convergence criteria.

The model structure and calibration parameters of the YRTP model were largely preserved in VSAM. For more details regarding modelling methodology and calibration parameters, please refer to the following documents prepared for the YRTP program:

- Ridership Forecasting Model Development Draft Report, July 2003
- Ridership Forecasting - Model Development Report: v1.1, Dec 2003
- YRTP Model User's Manual v1.11, Feb 2005


### 2.4 Model Update

### 2.4.1 Background

As a forecasting tool designed for transit service planning, the YRTP model was calibrated specifically to provide ridership forecasts during the AM peak period with traffic zone refinement focused primarily in the rapid transit corridors along Yonge Street and Highway 7. Given its coarse traffic zone system outside the designated rapid transit corridors and its inability to forecast PM traffic (when travel demand is highest due to additional discretionary trips on the local roads), the YRTP model is not suitable for generating detailed traffic and ridership data for the network assessment required by this TMP study. To address these specific issues, Halcrow developed a sub-area model, VSAM, based on the existing YRTP model to produce more reliable traffic forecasts in local development areas within Vaughan for both AM and PM periods. The following sections document the VSAM update process.
2.4.2 Traffic Zone Refinement

In consultation with City Staff and the study team, traffic zones within the City of Vaughan were refined to produce a more detailed network and a more precise depiction of walk distances to transit stations/stops. Centroid connectors for trips coming in and out of traffic zones were also adjusted to ensure appropriate vehicle loading to road network for these refined traffic zones. Total number of traffic zone within the City increased from 124 in the YRTP model to 185 in VSAM, with 61 new zones added as a part of the refinement process.

### 2.4.3 Demographics Update

2006 to 2031 population and employment estimates for the entire York Region at the 2001 TTS traffic zone level were provided by the Region's Planning Department for the use of this study. However, these numbers did not reflect the anticipated growth or intensification within the City, particularly for the focused study areas (e.g. Vaughan Metropolitan Centre, Vaughan Mills and Woodbridge Core areas). As such, additional land use re-allocation was undertaken by Halcrow in close collaboration with Urban Strategies Inc. and the City Staff, to incorporate the City's latest growth scenarios, to update demographics for each 2001 TTS traffic zone, and to disaggregate the demographic and other land use data for the VSAM refined traffic zones.

### 2.4.4 Auto and Transit Network Refinement

Network coding for the 2006 auto network within the City was validated using 2007 aerial photos available on the York Region Geomatics Branch website to ensure proper representation of road alignment and lane configuration ${ }^{1}$. Travel speeds and road capacities within the City were also vetted for reasonableness. Transit route alignment, stop location and frequency were verified using published schedules provided by York Region Transit (YRT) and GO Transit for all transit routes that traverse Vaughan. Selected local collector roads that are under the jurisdiction of the City were coded in the VSAM in consultation with City Staff to allow for analysis of local traffic diversion and infiltration. Volume delay functions for estimating travel times on 407ETR were updated to reflect 2006 toll costs and estimated value of time.

### 2.4.5 PM Model Development

In order to address the need for PM peak period forecasts (when traffic loads are the highest for the day), a PM model was developed to estimate base year and future PM peak hour auto traffic demand based on the calibrated AM model. This PM model was developed by a procedure that transposes and factors AM auto driver matrices to PM based on relationships between the AM and PM trips by purpose, as identified from the 2006 TTS data. More importantly, this new PM model has the capability to estimate local shopping or pass-by trips for shopping centres or retail stores based on the number retail jobs for each individual traffic zone. This feature greatly improves the robustness of the PM peak forecasting results as large numbers of discretionary trips are made during the afternoon peak period, which cannot be adequately accounted for with the transpose and factor method.

### 2.4.6 Model Validation and Fine-Tuning

AM and PM trip matrices estimated by the VSAM were compared to the observed 2006 TTS data, and assigned traffic volumes in the VSAM were validated against observed traffic counts for each individual screenline station. Based on these validation results, an adjustment matrix was developed, after successive testing, to account for differences between the forecasted and observed 2006 AM and PM auto driver trips. Effectively, this matrix reflects the variations in trip generation rates (from the average rates used by the model) that are observed for specific land uses within the study area. This refinement significantly improves the accuracy of the model in the base year and is applied to all horizon year scenarios.

The following sections describe the model inputs required for the forecasting process.

[^1]
### 2.5 Traffic Zone System and Demographics

### 2.5.1 Traffic Zone System

The standard 2001 TTS traffic zone system developed by DMG divides the entire GTHA into 1,717 traffic zones with 103 zones located within Vaughan. Some of these 103 traffic zones along Yonge St. and Highway 7 have been split in the previous YRTP Study, which increased the total number of traffic zones in Vaughan to 124 for the YRTP Model. However, the level of traffic zone detail in the YRTP model was not sufficient for the subarea modelling in Vaughan. A more refined sub-area zone system was therefore redefined in conjunction with the City, to better model the zonal access points throughout the city. As a result of this traffic zone refinement process, a total of 60 traffic zones were added in the VSAM, with the YRTP traffic zone system was maintained outside of Vaughan. The VSAM has a total of 1,885 traffic zones, with 184 zones in Vaughan, and 26 external traffic zones that represent trips coming from and to areas outside of the GTHA,. Table 1 shows the traffic zone number assigned for each zone group and Figure 1 illustrates the Vaughan sub-area zone system.

Table 1- VSAM Traffic Zone Numbering System

| VSAM Study Area | Tz \# Range |
| :--- | :---: |
| Vaughan | $1006-1103$ |
| Gaughan (VSAM Split) | $6001-6140$ |
| Tz Region | $1-481$ |
| City of Toronto | $501-765$ |
| Durham Region | $1006-1353$ |
| York Region | $6001-6319$ |
| York Region(VSAM Split) | $1501-1753$ |
| Peel Region | $2001-2197$ |
| Halton Region | $2501-2670$ |
| City of Hamilton | $4000-4410$ |
| External Zones |  |

Figure 1- VSAM Traffic Zone System Map


[^2]Land Use (Population and Employment)
Current and future year population and employment by traffic zone are key inputs to the VSAM model. For the base year (2006) VSAM, the latest population and employment data for the City of Vaughan provided by the City's Planning Department at the 2001 traffic zone level were utilized for the City, and the comparable data assumed by York Region for the YRTP model were adopted for areas outside Vaughan.

Since traffic zones within Vaughan have been refined in detail for better network representation, manual allocation were undertaken in collaboration with Urban Strategies Inc. and City Staff to distribute 2006 population and employment estimates at the 2001 traffic zone system to each smaller and refined zone. Split percentages used to distribute zonal employment into sub-categories (office / manufacturing / professional) in the YRTP model were retained for the VSAM. Table 2 summarizes the 2006 population and employment estimates for Vaughan and GTHA municipals.

Table 2-2006 Population and Employment

| Area | Pop | Emp |
| :--- | ---: | ---: |
| Toronto PD1 | 190,937 | 451,065 |
| Rest of Toronto | $2,405,601$ | $1,082,880$ |
| Durham | 588,935 | 194,375 |
| York | 929,865 | 459,152 |
| Peel | $1,205,877$ | 636,884 |
| Halton | 429,900 | 194,000 |
| Hamilton | 515,000 | 199,600 |
| GTHA | $\mathbf{6 , 2 6 6 , 1 1 5}$ | $\mathbf{3 , 2 1 7 , 9 5 6}$ |
| Vaughan | $\mathbf{1 5 8 , 9 9 9}$ |  |

### 2.6 Road and Transit Networks

2.6.1 Road Network

The road network in the YRTP model was first developed based on the 2001 GTA EMME/2 network provided by DMG. It was updated in 2006 by York Region staff for their modelling purposes and subsequently by Halcrow for the Western Vaughan IEA Study. While most of the network attributes (number of lanes, speed, capacity, length, turn restriction, etc.) from these previous models were preserved for VSAM, Halcrow conducted an extensive check and refinement of network coding within the Vaughan sub-area to ensure accurate network representation. Road geometry for the roadways within Vaughan was also refined to improve visual appearance by adding details to the highway interchanges and arterial roads. Selected local collector roads under the jurisdiction of the City were also added to the VSAM for local traffic diversion and infiltration analysis. The revised network is illustrated in Figure 2.

Figure 2 - 2006 VSAM Road Network


### 2.6.2 Transit Network

Similar to the road network, the transit network in the YRTP was first developed based on the 2001 GTA EMME/2 network with further updates by York Region staff and Halcrow. For this Vaughan TMP Study, Halcrow reviewed and updated the transit headways, route alignments and stops based on the latest schedule available for all the YRT, TTC and GO transit routes that traverse the City of Vaughan. Table 3 shows all the transit routes that have been verified and updated for VSAM.

Table 3 - VSAM AM Transit Network

| Transit Agency | Route \# | Route ID | Transit Agency | Route \# | Route ID |
| :---: | :---: | :---: | :---: | :---: | :---: |
| GO bus | 32 | Brampton - Sheppard | TTC | 165F | Weston Rd South F |
| GO bus | 42 | Bolton - York Mills | TTC | 191 | Hwy 27 Rocket |
| GO bus | 44 | Mount Joy - York U | TTC | Subway | Bloor - Danforth Subway |
| GO bus | 45 | Streetsville-York U | TTC | Subway | Finch - Downsview Subway |
| GO bus | 45A | York U - Square One | TTC | Subway | Scarborough RT |
| GO bus | 46 | Oakville - York U | TTC | Subway | Sheppard Subway |
| GO bus | 47 | Hamilton - York U | VIVA | Pink | VIVA Pink |
| GO bus | 48A | Meadowville - York U | VIVA | Purple | VIVA Purple |
| GO bus | 48B | Meadowville - York U | VIVA | Green | VIVA Green |
| GO bus | 49 | York U - Pickering | VIVA | Orange | VIVA Orange |
| GO bus | 49 | Pickering - York U | VIVA | Blue | VIVA Blue |
| GO bus | 49 | Pickering - York U 407 | YRT | 3 | Thornhill - Y ork U |
| GO bus | 52 | Oshawa-York U | YRT | 3B | Thornhill - YorkU |
| GO bus | 61 | Richmond Hill - Union | YRT | 4 | Major Mackenzie |
| GO bus | 62 | Newmarket - York Mills | YRT | 4A | Major Mackenzie |
| GO bus | 64 | Newmarket - York U | YRT | 5 | Clark |
| GO bus | 65 | Barrie - Maple | YRT | 7 | Martin Grove |
| GO bus | 65 | Newmarket - Union | YRT | 10 | Woodbridge - York U |
| GO bus | 65 | King City - Union | YRT | 11 | Woodbridge |
| GO bus | 66 | Newmarket - Yorkdale | YRT | 12 | Pine Valley |
| GO bus | 66 | Yorkdale - Newmarket | YRT | 13A | Islington NapaValley |
| GO bus | 68 | Bradford - Yorkdale | YRT | 13B | Islington Nashville |
| GO rail | 65 | Bradford - Yorkdale | YRT | 20 | Jane |
| TTC | 35A | Jane A | YRT | 22 | King City |
| TTC | 35C | Jane C | YRT | 23 | Thornhill Woods |
| TTC | 35D | Jane D | YRT | 27 | Highway 27 |
| TTC | 35E | Jane E | YRT | 77 | Hwy 7 / Centre |
| TTC | 37D | Islington North | YRT | 83 | Trench |
| TTC | 37 | Islington | YRT | 83A | Trench |
| TTC | 46 | Martin Grove | YRT | 85 | Rutherford - 16 Ave. |
| TTC | 60E | Steeles West E | YRT | 85A | Rutherford - 16 Ave |
| TTC | 60F | Steeles West F | YRT | 85B | Rutherford - 16 Ave |
| TTC | 84A | Sheppard West A | YRT | 86 | Weldrick Newkirk |
| TTC | 84D | Sheppard West D | YRT | 87 | Langstaff Maple |
| TTC | 105B | Dufferin North | YRT | 88 | Bathurst |
| TTC | 107B | Keele North | YRT | 90 | Leslie South |
| TTC | 107C | Keele North | YRT | 98 | Yonge North |
| TTC | 107F | Keele North | YRT | 99 | Yonge South |
| TTC | 160 | Bathurst North | YRT | 260 | Rutherford GO Shuttle |
| TTC | 165 | Weston Road North | YRT | 360 | Maple Express |
| TTC | 165D | Weston Road North D | YRT | 463 | Emily Carr Sec Sch |
| TTC | 165F | Weston Rd North F |  |  |  |

### 2.6.3 Volume Delay Functions

A volume delay function (vdf) estimates the link travel time (in minutes) as a function of the link length, number of lanes, free flow speed and road capacity. As traffic volume increases, travel speeds decline, resulting in higher travel times on the link. VSAM adopts the standard Bureau of Public Road (BPR) type of vdf's for arterial road and freeway from the YRTP model as shown in Equation 1 and Equation 2. The shape of the volume delay
function, which is determined by the calibration parameters $\alpha$ and $\beta$ functions, are plotted in Figure 3. Table 4 shows the typical capacity assumptions for each road class.

## Equation 1 - Travel Time for Arterial Road ( $\alpha=1.0$ and $\beta=4.0$ )

Auto Travel Time $=$ Length $\times \frac{60}{\text { FreeFlowSpeed }} \times\left\{1+1.0 \times\left(\frac{\text { Total Volume }}{\text { Lanes } \times \text { RoadCapacity }}\right)^{4.0}\right\}$

## Equation 2 - Travel Time for Freeway ( $\alpha=1.0$ and $\beta=6.0$ )

Auto TravelTime $=$ Length $\times \frac{60}{\text { FreeFlowSpeed }} \times\left\{1+1.0 \times\left(\frac{\text { Total Volume }}{\text { Lanes } \times \text { RoadCapacity }}\right)^{6.0}\right\}$

Figure 3 - Volume Delay Functions


Table 4 - Typical Capacity for Each Road Class

| Road Classification | Typical Capacity <br> (veh/hr/lane) |
| :--- | :---: |
| Freeways | 1,800 |
| Freeway ramps | 1,400 |
| Controlled access or rural highways \& arterial roads | $1,200-1,500$ |
| High capacity urban arterial roads | 900 |
| Medium capacity urban arterial roads | 700 |
| CBD/minor arterial and collector roads | 500 |
| Centroid Connectors | 9,999 |

### 2.6.4 407ETR Traffic Assignment

The 407ETR is the only toll road in Ontario and its usage is strongly influenced by toll rates and potential users' willingness to pay the associated costs. Typically, associated toll costs for a complete trip are included in the volume-delay function to model the usage of this toll facility. The resulting total travel cost is often called "generalized time", which is a combination of the actual driving time and the perceived toll charge in minutes. The YRTP model uses this generalized time in the trip distribution stage, but treats the 407ETR is the same as other 400 -series freeways in the last stage of trip assignment. This treatment would potentially lead to underestimation of travel demand on Vaughan's local road system since traffic would divert from severely congested alternative roads (e.g. Highway 7) to the 407ETR without considering the toll costs associated with the usage of the toll facility. This issue was addressed in the VSAM by applying generalized time assignment in both trip distribution and trip assignment stages. An updated toll charge of $14.72 \phi / \mathrm{km}$ and value of time of $\$ 22 / \mathrm{hr}$ were used to reflect 2006 values. A discount of 20 percent calculated on top of the toll cost is estimated based on recent toll road studies in North America to reflect the travel reliability gained by using the 407ETR. All these parameters have been refined and validated through an iterative calibration process and the traffic assignment was validated against the observed 407ETR traffic counts (provided to Vaughan by 407ETR staff) to ensure a reasonable goodness of fit. Equation 3 shows the volume-delay function used for estimating 407ETR travel time.

## Equation 3 - Generalized Travel Time for 407ETR ( $\alpha=1.0$ and $\beta=6.0$ )

## Auto TravelTimefor 407ETR=

Length $\times \frac{60}{\text { FreeFlowSpeed }} \times\left\{1+1.0 \times\left(\frac{\text { Total Volume }}{\text { Lanes } \times \text { RoadCapacty }}\right)^{6.0}\right\}+$ Length $\times \frac{\text { TollRate }}{\text { ValueofTime }} \times 0.8$

### 2.6.5 Transit Travel Time

The VSAM uses an average operating speed for each individual bus route to estimate the transit travel time. For transit routes that run on exclusive rights-of-ways (e.g. commuter rail, subways and rapid transit), transit travel times are used to estimate transit speeds on specific transit segments instead. However, as is the case with the YRTP model, transit travel time is not sensitive to congestion on auto networks as the travel speeds are hardcoded.

## Equation 4 - Transit Segment Travel Time for Transit with Explicit Rights-of-Way

Transit Segment Travel Time $=$ Length $\times \frac{60}{\text { TransitSegmentSpeed }}$

### 2.7 Model Inputs

2.7.1 Seeding Matrix and External Trips

Besides land use data and transportation networks, the VSAM also requires a set of input matrices that are prepared prior to a full model run. One of these input matrices is the auto driver "seeding" matrix, which is used at the first cycle of the trip distribution stage to initiate the gravity model for estimating the number of trips between O-D pairs. This seeding matrix contains the AM peak auto driver trip data collected from the 2006 TTS. Seeding is used only once throughout the model feedback loop and is overridden by the output trip matrix generated by the final trip assignment for the rest of the modelling cycles. The external auto driver trip matrix is another input matrix that is added to the internal trip matrix to include all the trips that would potentially travel on the VSAM transportation network. Both of these matrices were updated in this study to reflect the latest 2006 TTS data released by DMG in fall of 2008. The seeding and external matrices, together with other required input matrices inherited from the YRTP Model, have been split and allocated to the refined VSAM traffic zone system using the appropriate population and employment distributions. An annual growth rate of $1.5 \%$ is assumed for all external trips in the future horizon years.

### 2.7.2 Incremental Matrices

Given the complexity of model procedures and algorithms, the model coefficients and parameters were rigorously calibrated to replicate observed travel behaviour as close as possible. However, the model algorithms represent the average condition and are not able to reflect the subtle differences associated with the specific socio-demographic and/or land use characteristics in one area versus another (e.g., a Walmart may have higher trip generation rates than a Zellers, but both are retail stores). Like many regional models, the YRTP model provides reasonable travel demand estimates at the regional level but travel demand estimates are less than satisfactory for sub-areas like the City of Vaughan. To help solve this problem, an "incremental matrix" was introduced in the VSAM to enhance
the fit to observed auto traffic at key screenlines within Vaughan. The first step in the development of incremental matrices was to apply an iterative assignment and demand adjustment process. The initial model matrix was then subtracted from the demandadjusted matrices to create an "incremental matrix" for auto driver trips. This incremental matrix is subsequently added to the auto driver trip matrices prior to assignment for base and future years. This step significantly improves the base model validation and these adjustments are carried forward in future years. The final validation results are presented in the next chapter.

## 3 AM Model Validation

### 3.1 AM Trip Summary

Table 5 compares the 2006 VSAM AM peak period person trip totals with the 2006 TTS targets by travel mode. TTS data collected by DMG in 2006 shows that there are 2.6 million total trips travelling on GTHA network during AM peak period. Among these 2.6 million motorized trips, 1.75 million of the trip makers are auto drivers, 0.4 million are auto passengers and the remaining 0.5 million are transit passengers. The model generates very similar trip totals with less than 5 percent differences between the survey data and modelled trips. Regional transit mode share is estimated at $17 \%$ in VSAM compared to $18 \%$ from the survey.

Table 5-2006 AM Peak Period Person Trip Totals - GTHA

| Region | Mode | Orig Trips |  |  | Dest Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Survey* | Model | Diff | Survey* | Model | Diff |
| GTAH | Auto Driver | 1,732,290 | 1,818,390 | 5\% | 1,732,290 | 1,818,390 | 5\% |
|  | Auto Passenger | 383,760 | 388,820 | 1\% | 383,760 | 388,820 | 1\% |
|  | Transit | 467,890 | 463,640 | -1\% | 467,890 | 463,640 | -1\% |
|  | Total Trips | 2,583,940 | 2,670,850 | 3\% | 2,583,940 | 2,670,850 | 3\% |
|  | Transit Mode Share | 18\% | 17\% | -1\% | 18\% | 17\% | -1\% |

*- 2006 Transportation Tomorrow Survey (TTS) Data

For a smaller study area like Vaughan, however, model's goodness-of-fit is usually less accurate compared to the aggregated regional numbers due to difficulties in estimating local traffic variation and the unique travel characteristics for neighbourhoods. Table 6 shows that VSAM overestimates the travel demand by 6 percent for trips generated by Vaughan and 14 percent for trips attracted to Vaughan. To solve this problem without undergoing a major re-calibration effort to update the YRTP model parameters, an "incremental matrix" was introduced to enhance the fit to the observed auto traffic at key screenlines within Vaughan. This incremental matrix was estimated based on the latest 2006/2007 traffic counts on major arterials and local collectors within Vaughan. An iterative approach was undertaken to validate the matrix at both trip and screenline level. Table 7 shows the improved AM peak hour auto driver trips for trip assignment. Screenline validation is described in the next section.

Table 6-2006 AM Peak Period Person Trip Totals - Vaughan

| Region | Mode | Orig Trips |  |  | Dest Trips |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Survey* | Model | Diff | Survey* | Model | Diff |
| Vaughan | Auto Driver | 82,980 | 89,060 | 7\% | 88,500 | 101,250 | 14\% |
|  | Auto Passenger | 17,750 | 18,810 | 6\% | 17,850 | 18,970 | 6\% |
|  | Transit | 11,570 | 11,690 | 1\% | 5,010 | 6,440 | 29\% |
|  | Total Trips | 112,300 | 119,560 | 6\% | 111,360 | 126,660 | 14\% |
|  | Transit Mode Share | 10\% | 10\% | -1\% | 4\% | 5\% | 1\% |

*- 2006 Transportation Tomorrow Survey (TTS) Data

Table 7-2006 AM Peak Hour Auto Driver Trip Totals - Vaughan

| Region | Mode | Orig Trips |  |  |  | Dest Trips |  |  |  |
| :--- | :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Survey | Model | Diff | Survey | Model | Diff |  |  |
| Vaughan | Auto Driver | 35,670 | 36,220 | $2 \%$ | 38,170 | 38,870 | $2 \%$ |  |  |

### 3.2 Screenline Validation

Screenline validation was undertaken by comparing the observed traffic counts with the modelled volumes generated by VSAM. Figure 4 and Figure 5 show the 11 screenlines (22 directional screenlines) and 214 count stations identified for this validation to account for most of the major arterials and local collectors within Vaughan. ATR and turning movement counts were provided by York Region and the City of Vaughan while cordon counts from DMG and traffic counts from the 407 ETR were also utilized.

In measuring the goodness of fit for individual screenlines, the GEH statistic was used. A GEH analysis provides a different form of review and adds value because it considers the relative importance of specific roads or transit lines crossing each screenline in relation to the volume of traffic that they carry. For example, a $10 \%$ error on a count of 100 cars is less significant than a $10 \%$ error on a count of 3,000 cars.

The GEH statistic measures the overall level of error associated with traffic volumes on the individual roads being analyzed, with lower values reflecting a better fit between modelbased estimates and the observed traffic. A GEH statistic of less than 10 on individual screenlines is an accepted standard by international agencies (e.g. TransFund New Zealand) and, therefore, an effort has been made to achieve this standard for most, if not all screenlines. This statistic is defined as:
$G E H=\sqrt{\frac{\left(V_{\text {obs }}-V_{\text {est }}\right)^{2}}{0.5 \times\left(V_{\text {obs }}+V_{\text {est }}\right)}}$

Figure 4 - East-West Screenline


Figure 5 - North-South Screenline


Table 8 compares the modelled AM peak hour auto volumes with the observed traffic counts. 18 out of 22 directional screenlines ( $82 \%$ ) produce a GEH lower than 10 and approximately $77 \%$ of the auto count stations (165 out of 214 ) produce a GEH lower than 10.

Table 8 - AM Sub-Area Traffic Count Comparison

| Scln | Dir | Screenline | Obs | Est | GEH |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | N | N of King Vaughan Rd | 3,630 | 3,250 | 6.5 |
| 1 | S | N of King Vaughan Rd | 10,580 | 9,830 | 7.4 |
| 2 | N | N of Teston Rd | 4,970 | 4,430 | 7.8 |
| 2 | S | N of Teston Rd | 12,570 | 12,760 | 1.7 |
| 3 | N | N of Rutherford Rd | 7,170 | 6,490 | 8.2 |
| 3 | S | N of Rutherford Rd | 16,360 | 17,510 | 8.9 |
| 4 | N | N of Hwy 7 | 10,670 | 10,100 | 5.6 |
| 4 | S | N of Hwy 7 | 23,560 | 23,820 | 1.7 |
| 5 | N | N of Steeles Ave W | 13,540 | 11,950 | 14.1 |
| 5 | S | N of Steeles Ave W | 24,680 | 25,910 | 7.8 |
| 6 | E | E of Hwy 50 | 12,090 | 12,730 | 5.7 |
| 6 | W | E of Hwy 50 | 8,820 | 7,130 | 18.9 |
| 7 | E | E of Hwy 27 | 10,960 | 11,250 | 2.8 |
| 7 | W | E of Hwy 27 | 11,200 | 9,600 | 15.8 |
| 8 | E | E of Pine Valley Dr | 11,490 | 12,040 | 5.1 |
| 8 | W | E of Pine Valley Dr | 10,230 | 9,430 | 8.0 |
| 9 | E | E of Hwy 400 | 15,860 | 14,830 | 8.3 |
| 9 | W | E of Hwy 400 | 12,820 | 12,190 | 5.7 |
| 10 | E | E of Keele St | 7,920 | 8,550 | 7.0 |
| 10 | W | E of Keele St | 9,760 | 10,180 | 4.2 |
| 11 | E | W of Bathurst St | 10,470 | 11,510 | 9.9 |
| 11 | W | W of Bathurst St | 14,370 | 11,600 | 24.3 |

Figure 6 shows that the R-squared of the model fit to observed counts for all screenline stations is 0.97 . Based on these validation statistics, it can be concluded that the model provides reasonable travel demand estimation within the Vaughan sub-area.

Figure 6-2006 AM VSAM Screenline Validation


## 4 PM Model Development and Validation

### 4.1 Modelling Approach

To address the need to forecast auto traffic during the PM peak hour, conversion procedures were developed to transpose and factor the AM auto driver trip matrices to obtain the PM peak hour traffic. This chapter provides a summary of this conversion procedure, which involves the following steps:

- Analyze the 2006 TTS data and estimate conversion factors
- Introduce local shopping trip estimation
- Estimate and apply incremental matrices
- Validate the PM model at the screenline level


### 4.2 TTS Survey Data Analysis

### 4.2.1 Introduction

The TTS collected travel information from households within the GTHA and the survey data were used to develop the trip generation, trip distribution and mode choice model in the YRTP and VSAM AM models. Since the TTS database contains 24 -hour origin and destination information, the survey data were analyzed to understand relationship between the morning and afternoon trip purposes in the study area. Through the analysis of the TTS survey data, appropriate afternoon trip purposes were identified and compared to the morning trip purposes. Conversion factors were then developed to convert the AM auto driver trip matrices to the PM trip controls for each trip purpose. This section outlines the cross-tabulation conducted to obtain the conversion factors, followed by the validation of these conversion factors to ensure the consistency of the data.

### 4.2.2 PM Peak Period

It was necessary to determine the modelled PM peak period before establishing the AM to PM relationship. Table 9 shows the total number of GTHA destination trips that start from 3 pm till 7 pm by 3 -hours intervals. It is observed that higher demand normally occurs between 3 pm to 6 pm , which is a reasonable choice for the PM modelling period.

Table 9 - PM Peak Period Selection

| Trip Purpose of Destination | 2001 |  |  | 2006 |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 3:00-5:59PM | 3:30-6:29PM | 4:00-6:59PM | 3:00-5:59PM | 3:30-6:29PM | 4:00-6:59PM |
| Other | 345,100 | 392,630 | 445,000 | 366,190 | 410,590 | 458,430 |
| Subsequent work | 72,830 | 61,190 | 52,350 | 68,390 | 55,070 | 46,680 |
| School | 12,820 | 15,410 | 16,850 | 12,460 | 13,970 | 14,730 |
| Subsequent school | 5,080 | 5,210 | 5,450 | 4,740 | 5,330 | 6,060 |
| Daycare | 20,590 | 19,520 | 17,470 | 25,000 | 23,760 | 20,940 |
| Facilitate a passenger | 213,860 | 186,990 | 163,120 | 239,490 | 202,050 | 179,690 |
| Work | 104,050 | 85,430 | 72,330 | 96,670 | 78,850 | 66,090 |
| Home | 2,076,860 | 1,999,120 | 1,790,880 | 2,198,460 | 2,063,660 | 1,837,990 |
| Market/Shop | 208,240 | 217,030 | 229,200 | 237,980 | 240,680 | 247,510 |
| Unknown | 180 | 230 | 220 | 80 | 60 | 60 |
| Total | 3,059,610 | 2,982,760 | 2,792,870 | 3,249,460 | 3,094,020 | 2,878,180 |

### 4.2.3 PM Trip Purposes

The TTS survey data collected information according to 9 origin and 10 destination trip purposes that combined to produce over 90 unique trip purposes. For the purpose of model calibration, it was essential to consolidate these trip purposes into major categories with similar travel characteristics. Initially, the TTS data were aggregated into 14 main trip purposes that describe travel throughout the day. Table 10 shows the auto driver trip totals for the major trip purposes during the PM peak period (1500-1759). The 4 trip categories aggregated from these 12 main trip purposes for the VSAM AM Model are: to work, to secondary school, to post-secondary school and to other.

2006 trips by purpose were examined to understand PM travel patterns and to establish the relationship between AM and PM trip purposes. The cross tabulation analysis indicates that approximately 42 percent of the afternoon trips are related to work. The rest of the trips are mostly home-based other or non-home-based other trips. The linkage between AM "to work" trips and "work to home" trips in the PM is obvious. Less "work to home" trips are observed in the PM, which is logical as start time for work trips are usually more "peaked" during the AM period. For the rest of the trip purposes, however, relationships between AM and PM peak periods were less obvious.

Table 10 - Auto Driver Trip Purpose Analysis for PM Peak Period

|  |  | AM |  | PM |  |  |
| :--- | ---: | ---: | :--- | ---: | :--- | :--- |
| Trip Purpose | Trips |  | $\%$ | Trip Purp | Trips | $\%$ |
| To Work | 920,990 | $55 \%$ | To Work | 49,480 | $3 \%$ | Other (HBO \& NHBO) |
| From Work to Home | 27,460 | $2 \%$ | Other | 703,080 | $36 \%$ | From Work |
| To SS | 8,910 | $1 \%$ | To SS | 370 | $0 \%$ | Other (HBO \& NHBO) |
| SS to Home | 20 | $0 \%$ | Other | 5,110 | $0 \%$ | From SS |
| To PS | 23,700 | $1 \%$ | To PS | 5,240 | $0 \%$ | Other (HBO \& NHBO) |
| PS to Home | 60 | $0 \%$ | Other | 18,610 | $1 \%$ | From PS |
| To Other | 139,040 | $8 \%$ | Other | 288,170 | $15 \%$ | Other (HBO \& NHBO) |
| From Other to Home | 14,810 | $1 \%$ | Other | 157,530 | $8 \%$ | Other (HBO \& NHBO) |
| To Serve | 297,730 | $18 \%$ | Other | 161,680 | $8 \%$ | Other (HBO \& NHBO) |
| From Serve to Home | 95,310 | $6 \%$ | Other | 169,750 | $9 \%$ | Other (HBO \& NHBO) |
| Serve to Work | 109,860 | $7 \%$ | To Work | 1,230 | $0 \%$ | Other (HBO \& NHBO) |
| Work to Serve | 860 | $0 \%$ | Other | 65,050 | $3 \%$ | From Work |
| To Shop | 18,820 | $1 \%$ | Other | 168,160 | $9 \%$ | Other (HBO \& NHBO) |
| From Shop to Home | 3,090 | $0 \%$ | Other | 139,660 | $7 \%$ | Other (HBO \& NHBO) |
| Total | $\mathbf{1 , 6 6 0 , 6 6 0}$ |  |  | $\mathbf{1 , 9 3 3 , 1 2 0}$ |  |  |

As a result of this analysis, the 14 main trip purposes were re-aggregated for the afternoon to produce the following PM peak hour trip purposes as defined below:

- From Work - Based on the transposed and factored AM "to work" trips
- From Secondary School - Based on the transposed and factored AM "to SS" trips
- From Post Secondary School - Based on the transposed and factored AM "to PS" trips
- Other trips (Home-Based Other and Non-Home-Based Other) - Based on the AM "Other" trips


### 4.2.4 Conversion Factor of Auto Driver Trip Matrices

After selecting the modelled time period and trip purposes, conversion factors are to be estimated to adjust the transposed AM matrices to the PM control totals. Table 1 shows the aggregated auto driver trip totals for the AM and the PM peak periods. There are approximately $16 \%$ more trips in total during the PM period with double the amount of "other" trips observed during the PM. This is due to more discretionary trips being made in the afternoon.

Table 11 - Auto Driver Trip Totals by purpose (AM \& PM Periods)

| AM |  | PM |  |
| :--- | ---: | :--- | ---: |
| Trip Purpose | Trips | Trip Purpose | Trips |
| To Work | $1,030,860$ | From Work | 768,130 |
| To SS | 8,910 | From SS | 5,110 |
| To PS | 23,700 | From PS | 18,610 |
| Other | 597,170 | Other (HBO, NHBO) | $\mathbf{1 , 1 4 1 , 2 7 0}$ |
| Total | $\mathbf{1 , 6 6 0 , 6 4 0}$ | Total | $\mathbf{1 , 9 3 3 , 1 2 0}$ |

PM to AM Ratio:

Different super-zone systems have been tested to compute the conversion matrix. Table 12 shows the conversion factor set out for the conversion of AM work trips to PM. Table 13 shows the factors used for other trips.

Table 12 - AM to PM Conversion Factor for Auto Driver Work Trips

| Region | Toronto | Durham | York | Peel | Halton | Hamilton | total |
| :--- | ---: | :--- | :--- | :--- | :--- | :--- | :--- |
| Toronto | 0.7029 | 0.7930 | 0.7228 | 0.7387 | 0.7906 | 1.0623 | $\mathbf{0 . 7 2 2 0}$ |
| Durham | 0.6944 | 0.7428 | 0.8100 | 0.9468 | 1.8118 | 1.2000 | $\mathbf{0 . 7 4 4 8}$ |
| York | 0.7148 | 0.8160 | 0.7540 | 0.7752 | 0.6653 | 1.3989 | $\mathbf{0 . 7 4 9 7}$ |
| Peel | 0.7299 | 0.9610 | 0.7185 | 0.7773 | 0.7372 | 0.7317 | $\mathbf{0 . 7 6 1 3}$ |
| Halton | 0.7373 | 1.3569 | 0.6340 | 0.7942 | 0.7605 | 0.8221 | $\mathbf{0 . 7 7 7 9}$ |
| Hamilton | 0.6425 | 0.7687 | 0.6518 | 0.7215 | 0.7239 | 0.7807 | $\mathbf{0 . 7 7 2 2}$ |
| total | $\mathbf{0 . 7 0 8 2}$ | $\mathbf{0 . 7 7 2 0}$ | $\mathbf{0 . 7 3 8 8}$ | $\mathbf{0 . 7 6 9 4}$ | $\mathbf{0 . 7 5 2 6}$ | $\mathbf{0 . 7 9 6 3}$ | $\mathbf{0 . 7 4 5 1}$ |

Table 13 - AM to PM Conversion Factor for Auto Driver Other Trips

| Region | Toronto | Durham | York | Peel | Halton | Hamilton | Total |
| :--- | ---: | ---: | :--- | :--- | :--- | ---: | ---: |
| Toronto | 1.8713 | 2.2588 | 1.9919 | 2.1608 | 2.7671 | 1.9364 | $\mathbf{1 . 9 0 6 1}$ |
| Durham | 2.0273 | 1.9152 | 2.5229 | 1.4689 | 0.3684 | 1.1143 | $\mathbf{1 . 9 2 3 6}$ |
| York | 2.6550 | 2.4187 | 1.7264 | 2.4859 | 4.7727 | 3.2532 | $\mathbf{1 . 8 5 6 1}$ |
| Peel | 2.7541 | 2.3388 | 2.8792 | 1.6384 | 2.8996 | 3.3668 | $\mathbf{1 . 7 6 5 2}$ |
| Halton | 3.2396 | 1.4103 | 3.2967 | 2.6504 | 1.9980 | 2.8605 | $\mathbf{2 . 0 9 6 1}$ |
| Hamilton | 3.9177 | 1.9500 | 3.5179 | 1.5978 | 2.4829 | 2.2029 | $\mathbf{2 . 2 2 0 8}$ |
| Total | $\mathbf{1 . 9 5 1 9}$ | $\mathbf{1 . 9 4 7 2}$ | $\mathbf{1 . 7 9 8 1}$ | $\mathbf{1 . 7 1 6 3}$ | $\mathbf{2 . 1 0 7 0}$ | $\mathbf{2 . 2 4 2 7}$ | $\mathbf{1 . 9 1 1 1}$ |

To validate the conversion method, the control PM trip matrices cross-tabulated by the TTS data were compared against estimated PM matrices that were developed by transposing and factoring of the AM trip matrices at the sub-area level. Table 14 to Table 16 demonstrate the validation of the auto driver work trip matrix. Initially, TTS data were
cross-tabulated to generate an $A M$ trip matrix. The AM trip matrix was then transposed and factored to produce the output PM auto driver work trip matrix. Finally, this output PM auto driver work trip matrix was compared against the control PM trip matrix generated by cross-tabulation of TTS database. R-Squared of 0.99 as shown in Table 17 indicates a close fit between two matrices.
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| 2006 |  | ( |  | (10000 |  |  | (10) |  |  | \|cemen | (1) | ( | / | / | (1) | \% |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PD 1 | 5,650 | 7,140 | 250 | - | - | 70 | 100 | 270 | - | 870 | - | 480 | 2,790 | 250 | 160 | 18,030 |
| Rest of TO | 39,290 | 175,560 | 5,900 | 170 | 110 | 1,480 | 1,090 | 5,600 | 540 | 18,990 | 330 | 20,070 | 38,470 | 3,690 | 560 | 311,850 |
| Durham | 3,290 | 26,530 | 63,670 | 190 | 150 | 810 | 580 | 1,410 | 1,020 | 7,760 | 50 | 1,470 | 2,510 | 300 | 130 | 109,870 |
| Georgina | 180 | 980 | 220 | 1,680 | 320 | 2,090 | 600 | 490 | 140 | 850 | 20 | 340 | 200 | 60 | 20 | 8,190 |
| E Gwillimbury | 150 | 840 | 90 | 110 | 470 | 1,410 | 470 | 360 | 120 | 410 | 120 | 280 | 140 | - | - | 4,970 |
| NewMarket | 510 | 3,400 | 180 | 290 | 230 | 4,860 | 1,430 | 1,080 | 390 | 2,020 | 250 | 1,260 | 610 | 80 | 20 | 16,610 |
| Aurora | 470 | 2,820 | 90 | 60 | 110 | 1,200 | 1,880 | 1,080 | 110 | 1,520 | 230 | 1,020 | 710 | 90 | 20 | 11,410 |
| Richm ond Hill | 2,000 | 10,500 | 290 | 80 | 20 | 880 | 620 | 5,190 | 150 | 5,030 | 230 | 4,560 | 2,590 | 330 | 20 | 32,490 |
| Whitchurch Stouffville | 160 | 1,180 | 220 | 40 | 40 | 270 | 170 | 540 | 1,070 | 1,090 | 40 | 350 | 250 | - | - | 5,420 |
| Markham | 2,960 | 20,340 | 1,090 | 20 | 30 | 570 | 410 | 3,090 | 520 | 14,510 | 40 | 2,940 | 3,640 | 290 | 110 | 50,560 |
| King | 140 | 1,070 | - | - | 50 | 210 | 150 | 410 | 80 | 260 | 240 | 640 | 560 | 90 | - | 3,900 |
| Vaughan | 2,590 | 20,980 | 380 | 40 | 70 | 470 | 260 | 1,930 | 70 | 3,030 | 220 | 15,270 | 7,320 | 450 | 60 | 53,140 |
| Peel | 9,260 | 43,860 | 700 | - | 20 | 340 | 290 | 1,010 | 150 | 2,980 | 170 | 10,240 | 147,020 | 13,060 | 1,400 | 230,500 |
| Halton | 2,300 | 7,800 | 90 | - | 20 | 20 | 70 | 320 | 40 | 670 | - | 1,050 | 30,640 | 45,050 | 5,900 | 93,970 |
| Hamilton | 470 | 1,320 | 90 | - | - | - | 20 | 40 | 20 | 90 | - | 220 | 4,660 | 17,170 | 56,030 | 80, 130 |
| Total | 69,420 | 324,320 | 73,260 | 2,680 | 1,640 | 14,680 | 8,140 | 22,820 | 4,420 | 60,080 | 1,940 | 60,190 | 242,110 | 80,910 | 64,430 $\quad 1$ | 1,031,040 |

Table 15 - Estimated PM Auto Work Trip Matrix (Transposed and Factored AM Matrix)

| 2006 | cor |  | (1) | (100000 |  | \|cersin | (1) |  |  | \|cemen | ( |  |  |  |  | \% |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| PD 1 | 3,970 | 27,610 | 2,610 | 130 | 110 | 370 | 340 | 1,440 | 120 | 2,140 | 100 | 1,870 | 6,840 | 1,700 | 340 | 49,690 |
| Rest of TO | 5,020 | 123,410 | 21,030 | 700 | 610 | 2,460 | 2,040 | 7,590 | 850 | 14,700 | 770 | 15,160 | 32,400 | 5,760 | 970 | 233,470 |
| Durham | 170 | 4,100 | 47,290 | 180 | 70 | 140 | 70 | 240 | 180 | 880 | - | 310 | 660 | 80 | 90 | 54,460 |
| Georgina | - | 120 | 150 | 1,270 | 80 | 220 | 40 | 60 | 30 | 20 |  | 30 | - | - |  | 2,020 |
| E Gwillimbury | - | 80 | 120 | 240 | 350 | 170 | 80 | 10 | 30 | 30 | 40 | 60 | 10 | 10 | - | 1,230 |
| NewMarket | 50 | 1,060 | 660 | 1,580 | 1,070 | 3,660 | 900 | 660 | 200 | 430 | 160 | 350 | 260 | 10 | - | 11,050 |
| Aurora | 70 | 780 | 470 | 450 | 350 | 1,080 | 1,420 | 460 | 130 | 310 | 110 | 190 | 230 | 50 | 20 | 6,120 |
| Richmond Hill | 190 | 4,010 | 1,150 | 370 | 270 | 820 | 810 | 3,910 | 410 | 2,330 | 310 | 1,460 | 780 | 210 | 50 | 17,080 |
| Whitchurch Stouffville | - | 390 | 830 | 110 | 90 | 290 | 80 | 110 | 810 | 390 | 60 | 50 | 120 | 20 | 20 | 3,370 |
| Markham | 620 | 13,570 | 6,340 | 640 | 310 | 1,520 | 1,150 | 3,790 | 820 | 10,940 | 190 | 2,280 | 2,310 | 440 | 120 | 45,040 |
| King | - | 240 | 40 | 10 | 90 | 190 | 170 | 170 | 30 | 30 | 180 | 160 | 130 | - | - | 1,440 |
| Vaughan | 340 | 14,340 | 1,200 | 260 | 210 | 950 | 770 | 3,440 | 260 | 2,220 | 480 | 11,510 | 7,930 | 700 | 300 | 44,910 |
| Peel | 2,040 | 28,080 | 2,410 | 150 | 100 | 440 | 510 | 1,860 | 180 | 2,620 | 400 | 5,260 | 114,290 | 22,580 | 3,410 | 184,330 |
| Halton | 190 | 2,720 | 400 | 40 | - | 50 | 60 | 210 | - | 190 | 60 | 290 | 10,370 | 34,260 | 14,120 | 62,960 |
| Hamilton | 100 | 360 | 100 | 10 | - | 10 | 10 | 10 | - | 70 | - | 40 | 1,010 | 4,270 | 43,750 | 49,740 |
| Total | 12,760 | 220,870 | 84,800 | 6,140 | 3,710 | 12,370 | 8,450 | 23,960 | 4,050 | 37,300 | 2,860 | 39,020 | 177,340 | 70,090 | 63,190 | 766,910 |

## Table 14-2006 TTS AM Auto Driver Work Trips

## Table 16-2006 TTS PM Auto Driver Trip Control Totals


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### 4.3 Local Shopping Trips

### 4.3.1 Introduction

A satisfactory fit to regional travel data for the PM peak period is expected using the transpose and factor method. However, like many other macroscopic models, model fit on some local streets and commercial streets in the afternoon is less than satisfactory with substantial under-estimation of traffic. One of the reasons for this problem is the lack of shopping trips during the AM peak period (dominated by work and school-related travel), which means that very few if any shopping trip samples are collected in travel surveys during this period. Therefore the flip-and-factor approach cannot be used to capture the shopping trips generated in the afternoon. Also, it is a common problem in household travel surveys that short / by-pass trips and particularly "multiple-destination" shopping trips may not be reported at all. Furthermore, many local trips do not cross a regional screenline and may therefore not be part of the data to which the regional model like YRTP model is calibrated. To compensate for this under-representation of short-distance auto trips to/from shopping areas, a new shopping trip purpose was introduced to represent short trips to and from neighbouring shops or shopping centres. Such trips include homebased shopping as well as work-to-shop and shop-to-shop travel (a high proportion of which can be described as pass-by traffic where persons returning from work to home may stop on the way to pick up household items etc.).

### 4.3.2 Shopping Trip Estimation

A matrix growth factoring process was adopted to estimate PM shopping trips for Vaughan based on retail job ratios and shopping trip matrices extracted from the TTS database. As for the refined traffic zones where TTS retail job ratios cannot be applied directly, a set of split percentages developed with the aid of aerial photos was applied to distribute retail jobs to every refined zone in Vaughan. The resulting retail job estimates were used as control vectors in the factoring process to adjust the 2006 TTS "to shop" and "from shop" O-D matrices to reflect observed retail trips.

An iterative process was used to calibrate shopping trip attraction and production rates. After testing different sets of trip rates and validating against local screenlines and ITE trip rate targets, considering other trip purposes generated by shopping centres, PM peak hour shopping trip rates of 0.45 and 0.60 trips per retail job (for productions and attractions, respectively) were determined to be appropriate for estimating shopping trip trip generation associated with major shopping centres in Vaughan. These trip rates are to be used to forecast shopping trips for base and future horizon years for comparable shopping centres. It should be noted that the resulting shopping matrix also includes by-pass trips that were made by the "non-shoppers" as listed in the travel survey due to undercounting issue associated with "multiple-destination trips". These by-pass trips, which are now included in the new shopping trip matrix, can backed out from the non-shopping trip matrix using a calibrated factor of 0.80 .

### 4.3.3 Shopping Trips at Major Shopping Centres

The following six major shopping centres located in Vaughan were selected for validation purpose:

- RioCan Colossus Centre
- The Promenade
- The Interchange
- Westridge Power Centre
- Seven and 400 Power Centre
- Vaughan Mills

After attempts to collect traffic cordon counts for these major shopping centres, it was determined that traffic data were not sufficient for estimating total incoming and outgoing trips for the model base year. Therefore, Institute of Transportation Engineers (ITE) trip rates were used instead to estimate auto trip productions and attractions based on Gross Leasable Area (GLA) information provided by urbanMetrics inc. A comparison between trips associated with these locations and the nearby traffic counts indicate that ITE trip rates could be overstating the traffic to/from these shopping areas, especially for shopping centres along Highway 7, where frequent transit services and limited parking spaces might reduce auto usage. This issue is addressed by lowering the ITE trip "target" by $20 \%$ to reflect more reasonable trip generation rates for the study area. Table 18 summarizes the adjusted ITE trip totals for these six retail centres used for validation and the corresponding model forecasts.

Table 18 - ITE Trip Estimates for PM Peak Hour

| Location | Used GLA(sf) Retail | Category | ITE Trips* |  | VSAM Model |  | Diff vs. ITE |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  |  | IN | OUT | IN | OUT | IN | OUT |
| RioCan Colossus Centre | 572,600 | Retail | 760 | 820 | 770 | 860 | -9\% | -8\% |
|  | 113,100 | Theatre | 90 | 110 |  |  |  |  |
| The Promenade | 632,300 | Retail | 810 | 880 | 1,010 | 1,080 | 19\% | 15\% |
|  | 24,200 | Theatre | 40 | 60 |  |  |  |  |
| The Interchange | 510,200 | Retail | 700 | 760 | 810 | 980 | -5\% | 4\% |
|  | 111,000 | Theatre | 150 | 180 |  |  |  |  |
| Westridge Power Centre | 496,350 | Retail | 690 | 750 | 680 | 780 | -1\% | 4\% |
| Seven and 400 Power Centre | 330,600 | Retail | 670 | 720 | 480 | 680 | -28\% | -6\% |
| Vaughan Mills** | 1,336,350 | Retail | 1,330 | 1,440 | 1,370 | 1,550 | 3\% | 8\% |
| Overall |  |  | 5,240 | 5,720 | 5,120 | 5,930 | -2\% | 4\% |

*     - Based on adjusted ITE Trip Rates. ITE Trip Generation, 7th edition
** - Including Tuscany PI \& The Village


### 4.4 PM Model Validation

As with the AM VSAM model, an incremental matrix is developed to enhance the fit to the observed auto traffic at key screenlines during the PM peak period.

Table 19 provides a comparison of the model auto flow versus the observed PM peak hour auto counts for the 22 directional screenlines. It shows that 15 ( $68 \%$ ) out of 22 screenlines produce a GEH lower than 10 and the overall R-Squared is 0.96 as shown in Figure 7. Among all the 214 screenline stations, 160 of them ( $75 \%$ ) are within a GEH of 10. Based on these validation statistics, it can be concluded that the model provides reasonable travel demand estimation for the PM peak period within the Vaughan sub-area.

Table 19 - PM Sub-Area Traffic Count Comparison

| Scln | Dir | Screenline | Obs | Est | GEH |
| :---: | :---: | :---: | :---: | :---: | :---: |
| 1 | N | N of King Vaughan Rd | 11,030 | 10,570 | 4.4 |
| 1 | S | N of King Vaughan Rd | 3,900 | 3,810 | 1.3 |
| 2 | N | N of Teston Rd | 12,950 | 13,240 | 2.5 |
| 2 | S | N of Teston Rd | 5,190 | 5,280 | 1.2 |
| 3 | N | N of Rutherford Rd | 16,180 | 17,580 | 10.8 |
| 3 | S | N of Rutherford Rd | 7,180 | 7,870 | 8.0 |
| 4 | N | N of Hwy 7 | 21,810 | 23,470 | 11.0 |
| 4 | S | N of Hwy 7 | 14,180 | 12,810 | 11.8 |
| 5 | N | N of Steeles Ave W | 24,670 | 26,490 | 11.4 |
| 5 | S | N of Steeles Ave W | 16,320 | 15,560 | 6.0 |
| 6 | E | E of Hwy 50 | 8,790 | 7,650 | 12.6 |
| 6 | W | E of Hwy 50 | 11,370 | 12,440 | 9.8 |
| 7 | E | E of Hwy 27 | 11,210 | 10,600 | 5.8 |
| 7 | W | E of Hwy 27 | 11,640 | 11,900 | 2.4 |
| 8 | E | E of Pine Valley Dr | 11,510 | 12,280 | 7.0 |
| 8 | W | E of Pine Valley Dr | 12,220 | 13,450 | 10.9 |
| 9 | E | E of Hwy 400 | 14,890 | 15,560 | 5.4 |
| 9 | W | E of Hwy 400 | 17,000 | 17,210 | 1.6 |
| 10 | E | E of Keele St | 10,600 | 11,280 | 6.5 |
| 10 | W | E of Keele St | 8,250 | 9,170 | 9.8 |
| 11 | E | W of Bathurst St | 13,180 | 12,190 | 8.7 |
| 11 | W | W of Bathurst St | 9,860 | 11,640 | 17.2 |

Figure 7-2006 PM VSAM Screenline Validation


## 52031 Travel Demand Forecasts

### 5.1 Introduction

AM and PM travel demand forecasts were developed for the future 2031 base model. This chapter provides a summary of the following key model assumptions as well as the future year forecasts:

- Demographics
- Road and transit network improvements


### 5.2 Demographics

The Region of York is undertaking studies of future land use patterns in response to the Provincial "Place to Grow" Growth Plan, which calls for less greenfield development and greater intensification in transit-oriented development corridors and nodes. A set of demographics and land use projections that would conform to the Growth Plan was provided by the Region for this study in early 2009. Based on the Region's estimates, an update of the demographic data was undertaken by Urban Strategies Inc. and the City Staff to incorporate growth strategies that have been developed recently for Vaughan. Additional land use intensification in focused areas (e.g. Vaughan Metropolitan Centre (VMC) and the Vaughan Mills area) was also incorporated in the future base scenario. For areas outside of York Region, population and employment estimates assumed in the YRTP Model were retained in VSAM. Table 20 summarizes the population and employment projections for 2006 and 2031.

Table 20-2006 and 2031 Population and Employment Estimates

| Area | Population |  |  |  | Employment |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | 2006 | 2031 | Diff | CAGR | 2006 | 2031 | Diff | CAGR |
| Toronto PD1 | 190,940 | 266,170 | 39\% | 1.3\% | 451,070 | 568,610 | 26\% | 0.9\% |
| Rest of Toronto | 2,405,600 | 2,615,350 | 9\% | 0.3\% | 1,082,880 | 1,265,670 | 17\% | 0.6\% |
| Durham | 588,930 | 1,000,010 | 70\% | 2.1\% | 194,380 | 434,100 | 123\% | 3.3\% |
| York | 929,870 | 1,513,800 | 63\% | 2.0\% | 459,150 | 786,300 | 71\% | 2.2\% |
| Peel | 1,205,880 | 1,640,010 | 36\% | 1.2\% | 636,880 | 876,110 | 38\% | 1.3\% |
| Halton | 429,900 | 703,390 | 64\% | 2.0\% | 194,000 | 389,640 | 101\% | 2.8\% |
| Hamilton | 515,000 | 597,270 | 16\% | 0.6\% | 199,600 | 248,150 | 24\% | 0.9\% |
| GTHA | 6,266,120 | 8,335,990 | 33\% | 1.1\% | 3,217,960 | 4,568,570 | 42\% | 1.4\% |
| Vaughan | 248,810 | 425,150 | 71\% | 2.2\% | 159,000 | 262,800 | 65\% | 2.0\% |

### 5.3 Road and Transit Network Improvements

### 5.3.1 Road Network

Future road network improvements were provided by different agencies. Within Vaughan, proposed improvements for the local collector system were prepared by the City Staff while the latest regional arterial improvements assumed in the Western Vaughan IEA study were provided by the York Region. For areas outside Vaughan in York Region, the latest York Region Transportation Master Plan assumptions were coded into the 2031 base network while future network assumptions assumed in the YRTP Model are used for areas outside York Region. Table 21 identifies the key road network improvements for the 2031 base auto network assumed in VSAM.

Table 21-2031 Future Road Network Assumptions

| Within Vaughan |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| A. Provincial |  |  |  |  |  |  |
| \# | Roadway | From | To | Improvement | 2006 | 2031 |
| 1 | Hwy 50 | Steeles Ave. | Mayfield Rd. | Widen | $2 / 2$ | $3 / 3$ |
| 2 | Hwy 50 | Mayfield Rd. | Kirby Rd. | Widen | 1/1 | $3 / 3$ |
| 3 | Hwy 427 | NB off-ramp, SB on-ramp at Albion Rd. | Hwy 407 | Widen | 2/2 | $4 / 4$ |
| 4 | Hwy 427 | Hwy 407 | NB on-ramp, SB off-ramp at Hwy 407 | Widen | $2 / 3$ | 4/4 |
| 5 | Hwy 427 | Hwy 7 | Major Mac Dr. | Extension | - | $3 / 3$ |
| 6 | Hwy 400 | NB off-ramp, SB on-ramp at Steeles Ave. | Hwy 407 | Widen | 4/4 | 5/5 |
| 7 | Hwy 400 | Hwy 407 | Hwy 7 | Widen | 3/4 | 5/5 |
| 8 | Hwy 400 | NB off-/SB on-ramp at Bass Pro Mills Dr. | Rutherford Rd. | Widen | 5/4 | 5/5 |
| 9 | Hwy 400 | SB off-ramp, NB on-ramp at Rutherford Rd. | N of Rutherford Rd. | Widen | $4 / 5$ | 5/5 |
| 10 | Hwy 400 | NB off-ramp, SB on-ramp at Major Mac Dr. | Major Mac Dr. | Widen | $4 / 3$ | 5/5 |
| 11 | Hwy 400 | Major Mac Dr. | NB on-ramp, SB off-ramp at Major Mac Dr. | Widen | 3/3 | 5/5 |
| 12 | Hwy 400 | NB on-ramp, SB off-ramp at Major Mac Dr. | King-Vaughan Rd. | Widen | 3/3 | 4/4 |
| 13 | New Interchange | Martingrov | Rd. / Hwy 407 | $\begin{aligned} & \text { East-Oriented } \\ & \text { Partial IC } \\ & \hline \end{aligned}$ | - | - |
| 14 | New Interchange | Hwy 40 | Centre St. | Partial IC | - | - |
| 15 | Improved Interchange | Extended | wy 427 / Hwy 7 | Full | - | - |
| 16 | New Interchange | Extended Hwy | 27 / Langstaff Rd. | Full | - | - |
| 17 | New Interchange | Extended Hwy | 27 / Rutherford Rd. | Full | - | - |
| 18 | New Interchange | Extended Hwy | 27 / Major Mac Dr. | Full | - | - |
| B. Regional |  |  |  |  |  |  |
| \# | Roadway | From | To | Improvement | 2006 | 2031 |
| 19 | Hwy 27 | Steeles Ave. | Major Mac Dr. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 20 | Hwy 27 | Major Mac Dr. | Wilsen Rd. (King) | Widen | 1/1 | 2/2 |
| 21 | Pine Valley Dr. | Steeles Ave. | Hwy 7 | Hov | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 22 | Weston Rd. | Steeles Ave. | Major Mac Dr. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 23 | Weston Rd. | Major Mac Dr. | Kirby Rd. | Widen | 1/1 | 2/2 |
| 24 | Jane St. | Hwy 407 | Major Mac Dr. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 25 | Jane St. | Teston Rd. | King-Vaughan Rd. | Widen | 1/1 | 2/2 |
| 26 | Keele St. | Steeles Ave. | Major Mac Dr. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 27 | Dufferin St. | Steeles Ave. | Glen Shields Ave. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 28 | Dufferin St. | Major Mac Dr. | King Rd. | Widen | 1/1 | $2 / 2$ |
| 29 | Bathurst St. | Crestwood Rd. | Worth Blvd. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 30 | Bathurst St. | N of Autumn Hill Blva. | Elgin Mills Rd. | Hov | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 31 | King-Vaughan Rd. | Hwy 400 | Bathurst St. | Widen | 1/1 | 2/2 |
| 32 | Teston Rd. | Weston Rd. | E of Hwy 400 | Widen | 1/1 | $2 / 2$ |
| 33 | Teston Rd. | Keele St. | E of Rodinea Rd. | Widen | 1/1 | 2/2 |
| 34 | Teston Rd. | E of Rodinea Rd. | Dufferin St. | Extension | - | $2 / 2$ |
| 35 | Teston Rd. | Dufferin St. | Shaftsbury Ave. | Widen | 1/1 | 2/2 |
| 36 | Major Mac Dr. | Hwy 50 | W of Weston Rd. | Widen, HOV Jog elimination | 1/1 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 37 | Major Mac Dr. | W of Weston Rd. | McNaughton Rd. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 38 | Rutherford Rd. | Hwy 50 | Weston Rd. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 39 | Rutherford Rd. | Weston Rd. | Jane St. | Hov | 3/3 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 40 | Rutherford Rd. | Jane St. | Bathurst St. | HOV | 2/2 | $2 / 2 \mathrm{GP}+1 / 1$ |
| 41 | Langstaff Rd. | Hwy 50 | Hwy 27 | Widen | 1/1 | $2 / 2$ |
| 42 | Langstaff Rd. | Keele St. | Dufferin St. | Widen | 1/1 | 2/2 |
| 43 | Steeles Ave. | Weston Rd. | Jane St. | Widen | 2/2 | 3/3 |

Table 21 (con't) - 2031 Future Road Network Assumptions

| Within Vaughan |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| C. Cityof Vaughan |  |  |  |  |  |  |
| \# | Roadway | From | To | Improvement | 2006 | 2031 |
| 44 | Future E-W Rd., N of Major Mac Dr. | Hwy 50 | Huntington Rd. | New Link | - | $1 / 1$ |
| 45 | Huntington Rd. | McGillvray Rd. | Teston Rd. | Truncate + Realign | $1 / 1$ | $1 / 1$ |
| 46 | McGillvray Rd. | Rutherford Rd. | N of Rutherford Rd. | Realign | $1 / 1$ | $1 / 1$ |
| 47 | Future E-W Rd., N of Langstaff Rd. | Huntington Rd. | Hwy 27 | New Link | - | $1 / 1$ |
| 48 | *Zenway Blvd. | Old Huntington Rd. | Rainbow Creek Dr. | New Link | - | $1 / 1$ |
| 49 | Huntington Rd. | Fogal Rd. | N of Hwy 7 | Extension | - | $1 / 1$ |
| 50 | New Huntington Rd. | S of Langstaff Rd. | Hwy 7 | New Link | - | $2 / 2$ |
| 51 | Future E-W Rd., S of Langstaff | Old Huntington Rd. | New Huntington Rd. | New Link | - | 1/1 |
| 52 | Vaughan Valley Blvd. | Hwy 7 | Zenway Blvd. | New Link | - | 1/1 |
| 53 | Future E-W Rd., N of Kirby Rd. | Weston Rd. | Jane St. | New Link | - | $1 / 1$ |
| 54 | Future E-W Rd., N of Teston Rd. | Weston Rd. | Jane St. | New Link | - | 1/1 |
| 55 | Cityview Blvd. | Canada Dr. | Teston Rd. | Extension | - | $1 / 1$ |
| 56 | Future E-W Rd., N of Major Mac Dr. | Canada Dr. | America Ave. | New Link | - | $1 / 1$ |
| 57 | Future E-W Rd., N of Major Mac Dr. | Weston Rd. | Future N-S Rd. | New Link | - | $1 / 1$ |
| 58 | Future N-S Rd., E of Pine Vallye Dr. | Future E-W Rd. | Major Mac Dr. | New Link | - | 1/1 |
| 59 | Future N-S Rd., W of Weston Rd. | Future E-W Rd. | Major Mac Dr. | New Link | - | $1 / 1$ |
| 60 | Via Campanile | Davos Rd. | Major Mac Dr. | Extension | - | 1/1 |
| 61 | Davos Rd. | Via Campanile | Pine Valley Dr. | Extension | - | $1 / 1$ |
| 62 | Future N-S Rd., W of Hwy 400 | Creditview Rd. Terminus | Rutherford Rd. | New Link | - | 1/1 |
| 63 | Bass Pro Mills Dr. | Weston Rd. | Hwy 400 SB on-ramp | Extension | - | 1/1 |
| 64 | Bass Pro Mills Dr. | Romina Dr. | Jane St. | Extension | - | 1/1 |
| 65 | Peter Rupert Ave. | Rutherford Rd. | McNaughton Rd. | New Link | - | 1/1 |
| 66 | Maurier Blvd. | Peter Rupert Ave. | Dufferin St. | New Link | - | $1 / 1$ |
| 67 | Via Romano Blvd. | Major Mac Dr. | Teston Rd. | New Link | - | $1 / 1$ |
| 68 | Queen Filomena Ave. | Via Romano Blvd. | Bathurst St. | New Link | - | 1/1 |
| 69 | Thomas Cook Ave. | Rutherford Rd. | Major Mac Dr. | New Link | - | 1/1 |
| 70 | Lebovic Campus Dr. | Thomas Cook Ave. | Bathurst St. | New Link | - | $1 / 1$ |
| 71 | Pleasant Ridge Blva. | Apple Blossom Dr. | Rutherford Rd. | Extension | - | 1/1 |
| 72 | Future E-W Rd., N of Hwy 7 | Chrislea Rd. | Applewood Crescent | New Link | - | 2/2 |
| 73 | Fieldstone Dr. | Blue Willow Dr. | Weston Rd. | Widen | $1 / 1$ | $2 / 2$ |
| 74 | Applewood Cres. ( N of Hwy 7) | Applewood Cres. | Jane St. | Widen | $1 / 1$ | $2 / 2$ |
| 75 | Future Ring Rd. | E of Hwy 400 Off-ramp | Maplecrete Rd. | New Links | - | $2 / 2$ |
| 76 | Future N-S Rd. | Interchange Way | Ring Rd. | New Link | - | 2/2 |
| 77 | Future N-S Rd. | Hwy 7 | Chrislea Rd. | New Link | - | 2/2 |
| 78 | Future N-S Rd. | Hwy 400 NB off-ramp | Future Hwy 400 NB on-ramp | New Link | - | 1/2 |
| 79 | Future N -S Rd. | Future Hwy 400 NB on-ramp | Applewood Crescent | New Link | - | $2 / 2$ |
| 80 | Famous Ave. | Weston Rd. | Hwy 7 | Widen | 1/1 | $2 / 2$ |
| 81 | Credistone Rd. | MacIntosh Blvd. | N of Hwy 407 | Widen | $1 / 1$ | 2/2 |
|  |  |  | ghan, in GTHA |  |  |  |
| D. Provincial |  |  |  |  |  |  |
| \# | Roadway | From | To | Improvement | 2006 | 2031 |
| 81 | Hwy 407 | Hwy 401 (Peel) | Hwy 427 | Widen | $3 / 3$ | 5/5 |
| 82 | Hwy 407 | Weston Rd. | Hwy 404 | Widen | 4/4 | 5/5 |
| 83 | Hwy 407 | Hwy 404 | Kennedy Rd. | Widen | $3 / 4$ | $5 / 5$ |
| 84 | Hwy 407 | Kennedy Rd. | Markham Rd. | Widen | 3/3 | 5/5 |
| 85 | Hwy 407 | Markham Rd. | Brock Rd. (Durham) | Widen | $2 / 2$ | 5/5 |
| 86 | Hwy 407 | Brock Rd. | Hwy 401 (Oshawa) | Extension via Whitby | - | $2 / 2$ |
| 87 | Hwy 407 | Britannia Rd. West | Hwy 403 | HOV | $2 / 2$ | $2 / 2 \mathrm{GP}+1 / 1$ |
| 88 | Hwy 401 | Guelph Line | Trafalgar Rd. | HOV | 3/3 | $3 / 3 \mathrm{GP}+1 / 1$ |
| 89 | Hwy 401 | Trafalgar Rd. | Winston Churchill Blvd. | Widen + HOV | 3/3 | $5 / 5 \mathrm{GP}+1 / 1$ |
| 90 | Hwy 401 | Winston Churchill Blvd. | Hwy 410 | Widen + HOV | 3/3-4/4 | $6 / 6 \mathrm{GP}+1 / 1$ |
| 91 | Hwy 401 | Brock Rd. | Regional Rd. 34 / Courtice Rd | Widen + HOV | 3/3-5/5 | $6 / 6 \mathrm{GP}+1 / 1$ |

### 5.3.2 Transit Network

Transit network assumptions were coded based on the latest preferred option developed for the Western Vaughan IEA study which includes significant improvement in headway for
most of the bus routes that pass through Vaughan. Other key transit investments in the study area include:

- Bolton GO Rail
- TTC Spadina Subway Extension
- TTC Yonge Subway Extension
- 407 Transitway
- Improve transit headway for YRT VIVA line to 2-4 min
- Acceleride from Brampton to VMC


### 5.4 2031 Base Case Forecasts

### 5.4.1 AM \& PM Trip Summary

Table 22 presents the 2006 and 2031 trip totals for Vaughan and GTHA. In 2006, origin and destination trip totals for Vaughan were approximately 119,600 and 126,700 respectively. In 2031, trips originating from Vaughan is forecast to grow to 205,300 (72\% growth), while destination trip total is estimated at 203,900 (61\% growth). These high growth rates of total trips can be explained by the similar growth rates of population (71\%) and employment (65\%) in Vaughan as shown in Table 20. These growth rates also highlight the fact that future road and transit network within the study area is expected to accommodate almost double amount of travel demand in 2031.

Table 22 - AM Peak Period Trip Forecasts (2006 and 2031)

| Region | Mode | Orig Trips |  |  |  | Dest Trips |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2006 | 2031 | Diff | CAGR | 2006 | 2031 | Diff | CAGR |
| GTHA | Auto Driver | 1,818,390 | 2,470,240 | 36\% | 1.2\% | 1,818,390 | 2,470,240 | 36\% | 1.2\% |
|  | Auto Passenger | 388,820 | 517,260 | 33\% | 1.1\% | 388,820 | 517,260 | 33\% | 1.1\% |
|  | Transit | 463,640 | 633,040 | 37\% | 1.3\% | 463,640 | 633,040 | 37\% | 1.3\% |
|  | Total Trips | 2,670,850 | 3,620,540 | 36\% | 1.2\% | 2,670,850 | 3,620,540 | 36\% | 1.2\% |
|  | Transit Mode Share | 17\% | 17\% | 0\% | 0.0\% | 17\% | 17\% | 0\% | 0.0\% |
| Vaughan | Auto Driver | 89,060 | 143,480 | 61\% | 1.9\% | 101,250 | 159,790 | 58\% | 1.8\% |
|  | Auto Passenger | 18,810 | 31,210 | 66\% | 2.0\% | 18,970 | 29,970 | 58\% | 1.8\% |
|  | Transit | 11,690 | 30,640 | 162\% | 3.9\% | 6,440 | 14,090 | 119\% | 3.2\% |
|  | Total Trips | 119,560 | 205,330 | 72\% | 2.2\% | 126,660 | 203,850 | 61\% | 1.9\% |
|  | Transit Mode Share | 10\% | 15\% | 5\% | 1.7\% | 5\% | 7\% | 2\% | 1.2\% |

The growth of transit users is anticipated to be much faster than the auto drivers, as shown by the 5 percent increase of ridership for origin trips and 2 percent increase for destination trips. This increased transit mode share is mostly due to significant heavy transit service improvements, including the Spadina subway extension to VMC, planned improvement of transit frequency of the VIVA bus rapid bus, introduction of Bolton GO rail and also the improved services for other YRT bus routes as defined in the Western Vaughan IEA study.

The expected traffic growth rates in Vaughan are relatively higher than the rest of the GTA regions.

Table 23 summarizes the AM and PM peak hour traffic forecasts that are used for trip network assignment. Annual traffic growth rate for AM peak hour trips is 2 percent, which is reasonably close to the growth rate of $1.9 \%$ for the AM peak period while PM peak hour trips would grow at the similar rate of $1.7 \%$ per annum.

Table 23 - AM and PM Peak Hour Traffic Forecasts (2006 to 2031)

| Mode | Orig Trips |  |  |  | Dest Trips |  |  |  |  |
| :--- | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  | $\mathbf{2 0 0 6}$ | $\mathbf{2 0 3 1}$ | Diff | CAGR | $\mathbf{2 0 0 6}$ | $\mathbf{2 0 3 1}$ | Diff | CAGR |  |
| AM Auto Driver | 36,220 | 60,140 | $66 \%$ | $2.0 \%$ | 38,870 | 64,410 | $66 \%$ | $2.0 \%$ |  |
| PM Auto Driver | 58,070 | 89,100 | $53 \%$ | $1.7 \%$ | 55,300 | 84,680 | $53 \%$ | $1.7 \%$ |  |

Detailed forecasts for each focused area (e.g. VMC) required for this Vaughan TMP study are documented separately by Halcrow.

## 6 Summary and Conclusions

This report describes the model update and validation process of the AM and PM Vaughan Sub-Area Model (VSAM) developed for the Vaughan Transportation Master Plan (TMP) Study. The validation results presented show that VSAM generates reasonable and acceptable 2006 traffic volume estimates at both regional and local levels for both the AM and PM peak hours as documented in Sections 3 and 4.

The new PM VSAM model, which now has the capability of estimating total PM peak traffic including local shopping trips, produces reasonable traffic forecasts at and near major shopping centres during the afternoon peak period when auto traffic volumes are highest.

Traffic forecasts for the 2031 base land use/transportation system scenario indicate that the model is sensitive to the extensive transit investment planned for Vaughan, particularly for the VMC focused areas where aggressive land use intensification is planned. This model is thus applicable for the Vaughan TMP Study and is a practical tool to evaluate the City's future transportation infrastructure requirements that can be expected to encourage increased public transit use, alleviate congestion and promote livable street networks and neighbourhoods.

Other deliverables, including detailed transportation analysis for the designated focused areas and the final transportation model package, will be provided separately as the study proceeds.

F1.2 July 6, 2010 Memorandum

# Halcrow 

## Memo

| To | Dick G ordon | Project | Vaughan Transportation Master <br> Plan (update to Feb 10 Memo) |
| :--- | :--- | :--- | :--- |
| From | Halcrow Consulting Inc | Project no. |  |
| Date | 6 July 2010 | Re |  |

Copy Leah Russell, Lisa Wang, Mahboobeh Sohi

## VSAM 2031-Revised Base (AM \& PM), Altemative 1 (AM \& PM)

Further to discussions with the City of Vaughan and AECOM at a meeting on December 10, 2009, HCI has updated the Vaughan Sub-Area Model (VSAM) 2031 base network (developed November 2009) by incorporating the following revised inputs within the City of Vaughan study area:

- Land Use: population and employment
o source: Urban Strategies Inc. January 8, 2010
- Auto Network: number of lanes, capacity, and alignment
o source: revised Y ork Rapid Transit Plan (Y RTP model), July 24, 2009;
o source: Peel Region model, January 13, 2010
o source: Urban Strategies, January 20, 2010
- Transit Network: headway, speeds, alignment, station locations, and related access links
o source: base transit network as per Y RTP model provided by York Region for Western Vaughan IEA, August, 172007.
o source: updated transit network as per Y RTP model provided by York Region (used for changes in VSAM study area), D ecember 11, 2009.
o source: GO Transit, December 16, 2009.
o source: Y ork Region Transportation Master Plan Update, November 2009
- Parking: parking costs and locations
o source: revised Y RTP model, D ecember 11, 2009

The 2031 Alternative 1 scenario was developed based on this 2031 revised VSAM base, with the same assumptions in land use, transit network, and parking costs. Adjustments were made to the auto network with guidance from AECO M (February 22, 2010), i.e. regarding number of lanes, capacity, and alignment. This tech memo will address the 2031 revised VSAM base (a.m. and p.m.), the 2031 Alternative 1 (a.m. and p.m.), and the 2021 base (a.m. and p.m.).

# Híalcrow 

## Memo

## L and Use

Population and employment figures within the City of Vaughan were revised by Urban Strategies Inc. to match the land use control totals for Vaughan provided by York Region on December 11, 2009. Figure 1 and Figure 2 show the location and magnitude of the population and employment differences between the original VSAM 2031 base (November 2009) and the updated VSAM 2031 base (January 2010), respectively. The resultant population and employment totals can be seen in Table 1 below.

Figure 1-Population Differences
(Rev. VSAM 2031 Base versus Orig. VSAM 2031 Base)


[^3]
## Memo

Figure 2-Employment Differences
(Rev. VSAM 2031 Base versus Orig. VSAM 2031 Base)


2031 Employment Differences

Table 1-2031 Population and Employment City of Vaughan Totals

| VSAM Version | 2031 Population Totals | 2031 E mployment Totals |
| :--- | :---: | :---: |
| November 9, 2009 | 425,150 | $262,802^{*}$ |
| January 8, 2010 | 418,980 | $262,581^{*}$ |

*Adjustments were made for employment in Wonderland to reflect
average fall weekday traffic (TZ 1059)

# Halcrow 

## Memo

York Region also provided population and employment control totals for the rest of York Region outside of the City of Vaughan. While the overall totals for population did not change, the allocation by zone did change in some regions. The employment control totals were revised for all York Region municipalities. These updated GTHA 2031 forecasts were included in the revised 2031 VSAM base and the 2031 Alternative 1. Table 2 below illustrates the control totals outside of the City of Vaughan within York Region for both the original VSAM 2031 base (November 2009) and the revised VSAM 2031 base (January 2010).

Table 2-Population and Employment Totals outside Vaughan Orig. and Rev. VSAM 2031 Base

| Region |  | $\mathbf{2 0 3 1}$ Population |  | 2031 Employment |  |
| :--- | ---: | ---: | ---: | ---: | :---: |
|  |  | VSAM <br> Jan $\mathbf{8 , 2 0 1 0}$ | VSAM <br> Nov 9, 2009 | VSAM <br> Jan 8, 2010 |  |
| Aurora | 70,449 | 70,449 | 34,800 | 34,203 |  |
| East G willimbury | 87,997 | 87,997 | 31,738 | 34,412 |  |
| G eorgina | 70,691 | 70,691 | 21,721 | 21,244 |  |
| King | 35,095 | 35,095 | 12,111 | 11,915 |  |
| Markham | 423,468 | 423,468 | 248,936 | 240,572 |  |
| Newmarket | 97,346 | 97,346 | 49,647 | 49,445 |  |
| Richmond Hill | 242,816 | 242,816 | 100,749 | 99,388 |  |
| Whitchurch-Stouffville | 60,792 | 60,792 | 23,800 | 23,034 |  |
| York Region* | $\mathbf{1 , 5 0 7 , 4 8 0}$ | $\mathbf{1 , 5 0 7 , 4 8 0}$ | $\mathbf{7 9 9 , 7 4 8}$ | $\mathbf{7 8 0 , 2 6 7}$ |  |

*Totals include Vaughan
Retail E mployment
Retail employment for the 2006 base was generated using 2006 Transportation Tomorrow Survey (TTS) employment data. In zones that had been further split by HCI, the retail employment was allocated based on the location of retail development. Retail employment for each traffic zone was then expressed as a percentage of a total employment in each zone. Necessary adjustments to the percentages were made to ensure that the six major shopping centres located in Vaughan (RioCan Colossus Centre, The Promenade, The Interchange, Westridge Power Centre, Seven and 400 Power Centre, and Vaughan Mills) had the appropriate retail employment allocated for their respective traffic zones .

Subsequent to the calibration exercise, Urban Strategies provided (on a zonal basis) 2031 retail employment growth for the Vaughan Metropolitan Centre (VMC) area as well as 2031 total employment for the VSAM study area.

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The retail percentages per zone developed from the 2006 base were applied to the 2031 total employment to generate 2031 retail employment, while ensuring that the retail employment estimates in the VMC area were consistent with the employment growth figures provided by USI for VMC.

## A uto Network

Updated auto network assumptions regarding number of lanes, capacity, and alignments were adopted from the YRTP auto network (updated July 24, 2009) into the revised VSAM model for the City of Vaughan and in neighbouring municipalities where they were expected to have a significant impact on traffic patterns within the City of Vaughan. In addition, network refinements within the West Vaughan Secondary Employment Centre along the Highway 427 corridor were incorporated into the revised VSAM network based on recommendations from Urban Strategies. Table 3 and the accompanying Figure 3 provide details regarding the nature and the location of these updates within the Study area. Table 4 and Figure 4 provide details regarding network updates in Peel Region.

Table 3 - Auto network updates/ refinements
(Orig. VSAM 2031 Base and Rev. VSAM 2031 Base)

| ref | Revision | Orig. Base |  | New Base |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Lane | Cap | Lane | Cap |
| 1 | New NS road | na | na | $1 / 1$ | 400 |
| 2 | New NS road | na | na | $1 / 1$ | 500 |
| 3 | Capacity change |  | 800 |  | 700 |
| 4 | Lane/Capacity Change | $1 / 1$ | 400 | $2 / 2$ | 900 |
| 5 | Add EW road | na | na | $1 / 1$ | 400 |
| 6 | Lane/Capacity Change | $1 / 1$ | 600 | $2 / 2$ | 800 |
| 7 | Lane Change for SB lane | 4 |  | 5 |  |
| 8 | Lane Change for NS lane | 6 |  | 5 |  |
| 9 | Add NS road | na | na | $1 / 1$ | 600 |
| 10 | Add EW road | na | na | $1 / 1$ | 600 |
| 11 | Lane/Capacity Change | $2 / 2$ | 1000 | $1 / 1$ | 800 |
| 12 | Lane/Capacity Change | $3 / 3$ | 800 | $2 / 2$ | 900 |
| 13 | Lane Change | $2 / 2$ |  | $1 / 1$ |  |
| 14 | Lane Change | $2 / 2$ |  | $1 / 1$ |  |
| 15 | Lane/Capacity Change | $3 / 3$ | 800 | $2 / 2$ | 900 |
| 16 | Lane Change | $3 / 3$ |  | $2 / 2$ |  |
| 17 | Lane Change | $3 / 3$ | 800 | $2 / 2$ | 800,900 |

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Figure 3- Location of Auto N etwork Updates
(Orig. VSAM 2031 Base versus Rev. VSAM 2031 Base)


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## Table 4 - Auto N etwork Updates/ Refinements in Peel Region (Orig. VSAM 2031 Base and Rev. VSAM 2031 Base)

| ref | Revision | Orig. Base | New Base |
| :---: | :---: | :---: | :---: |
|  |  | Lane | Lane |
| 1 | Lane Change | 2/2 | 3/3 |
| 2 | Lane Change | 2/1 | 1/1 |
| 3 | Add EW road | na | 1/1 |
| 4 | Lane Change | 2/2 | 1/1 |
| 5 | Lane Change | 2/2 | 3/3 |
| 6 | Lane Change | 1/1 | 2/2 |
| 7 | Lane Change | 1/1 | 2/2 |
| 8 | Lane Change | 1/1 | 2/2 |
| 9 | Lane Change | 2/2 | 3/3 |
| 10 | Remove NS Road | 2/2 | na |
| 11 | Lane Change | 1/1 | 2/2 |
| 12 | Lane Change | 1/1 | 2/2 |
| 13 | Lane Change | 2/2 | 3/3 |
| 14 | Lane Change | 1/1 | 2/2 |
| 15 | Lane Change | 1/1 | 2/2 |
| 16 | Lane Change | 2/2 | 3/3 |
| 17 | Lane Change | 1/1 | 2/2 |
| 18 | Lane Change | 3/3 | 2/2 |
| 19 | Lane Change | 1/1 | 2/2 |
| 20 | Lane Change | 3/3 | 2/2 |
| 21 | Lane Change | 2/2 | 1/1 |
| 22 | Lane Change | 2/2 | 3/3 |
| 23 | Lane Change | 2/2 | 1/1 |
| 24 | Lane Change | 2/2 | 3/3 |
| 25 | Lane Change | 2/2 | 3/3 |
| 26 | Lane Change | 1/1 | 2/2 |
| 27 | Lane Change | 3/3 | 2/2 |
| 28 | Lane Change | 1/1 | 2/2 |
| 29 | Lane Change | 2/2 | 3/3 |
| 30 | Lane Change | 2/2 | 3/3 |
| 31 | Lane Change | 1/1 | 2/2 |
| 32 | Lane Change | 3/3 | 2/2 |
| 33 | Lane Change | 2/2 | 3/3 |
| 34 | Lane Change | 1/1 | 3/3 |
| 35 | Lane Change | 3/3 | 2/2 |
| 36 | Lane Change | 4/4 + 1/1 HOV | $3 / 3+1 / 1 \mathrm{HOV}$ |
| 37 | Lane Change | 2/2 | 3/3 |
| 38 | Lane Change | 3/3 | 2/2 |
| 39 | Lane Change | 3/3 | 2/2 |
| 40 | Lane Change | 2/2 | 3/3 |

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Figure 4 - Location of Auto Network Updates in Peel Region (Orig. VSAM 2031 Base versus Rev. VSAM 2031 Base)


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## Transit Serviœ

Service frequencies and speeds of transit routes traversing the City of Vaughan were revised by HCI primarily based on WVIEA version 8G assumptions adjusted to reflect the Region's updated TMP. In some cases, such as Major Mackenzie Dr., west of Weston Road, bus speeds were adjusted downward to reflect forecast traffic conditions, and headways were reduced from the Region's recent assumptions to reflect experience in comparable mature urban areas. The alignment of the Bolton GO line was adjusted to reflect the latest Y ork Region TMP update document (November 2009) and the locations of future stations were confirmed with GO Transit staff. Furthermore, walk access link assumptions for the revised transit lines were adjusted accordingly. Details regarding the changes in headway, service frequency, route alignment, and station locations can be seen in Table 5 and the associated Figure 5.

Table 5 - Transit Service Updates
(Orig. VSAM 2031 Base versus Rev. VSAM 2031 Base)

| Line Name | Description | Headway |  | Speed |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | $\begin{gathered} \text { Ori. } \\ \text { VSAM } \end{gathered}$ | $\begin{gathered} \text { Rev. } \\ \text { VSAM } \end{gathered}$ | $\begin{gathered} \text { Ori. } \\ \text { VSAM } \end{gathered}$ | $\begin{gathered} \text { Rev. } \\ \text { VSAM } \end{gathered}$ |
| T105XN | Dufferin North NB | 5 |  | 28 | 15 |
| T105XS | Dufferin North SB | 5 |  | 28 | 15 |
| Y004AE | Major Mackenzie EB | 10 |  | 37.6 | 24 |
| Y 004AW | Major Mackenzie WB | 10 |  | 37.6 | 24 |
| Y004B | Major Mackenzie Extension | 3 | 7.5 | 37.6 | 27.6 |
| Y004LE | Major Mackenzie BRT EB | na | 5 | na | 27 |
| Y004LW | Major Mackenzie BRT WB | na | 5 | na | 27 |
| Y022N | King City NB | 15 |  | 36 | 25 |
| Y022S | King City SB | 15 |  | 36 | 25 |
| Y 085 | Rutherford-16 ${ }^{\text {th }}$ | 3 | 8 | 25.1 |  |
| Y087E | Langstaff Maple EB | 30 |  | 31.8 | 19.8 |
| Y 087W | Langstaff Maple WB | 25 |  | 31.8 | 19.8 |
| G 9008 ${ }^{1}$ | Bolton GO line | 20 |  | 99 |  |

${ }^{1}$ Station locations revised

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Figure 5 - Revised Transit Lines in Revised VSAM 2031 Base


Parking
York Region provided revised parking costs (D ecember 11, 2009) that were included in revised the 2031 VSAM base network. The resultant difference between the original VSAM 2031 base (November, 2009) and the revised VSAM 2031 base (January 2010) were the additional parking locations (along with associated daily parking costs) shown in Figure 6.

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Figure 6 - Location of Parking Updates (Rev. VSAM 2031 Base versus Orig. VSAM 2031 Base)


## Base AM Model Results

The model results package for the revised VSAM 2031 base (Scenario 50813) including auto volume, auto and truck volume, v/c ratio (with and without trucks), transit ridership, and various network attributes can be found in Appendix A.

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## A uto V olumes

In order to assess the effects of land use on auto volumes separate from auto and transit network changes and parking costs. Comparisons were made between scenarios with alternate network and land use assumptions:

|  | Orig. LU | New LU |
| :---: | :---: | :---: |
| Orig. Network | Sc40013 | Sc40113 |
| New Network | Sc50713 | Sc50813 |

In order to isolate the effects of land use changes, Scenario 50813 (new land use, new auto and transit network) was compared against Scenario 50713 (original land use, new auto and transit network). To evaluate the impacts of network changes, Scenario 50813 (new land use, new auto and transit network) was compared against 40113 (new land use, original auto and transit network).

## L and U se Impacts

Overall, the differences in auto volumes within the City of Vaughan shown in Figure 7 correlate with the changes made in land use shown in Figure 1 and Figure 2. The green colour in the following figures indicates a reduction in traffic in the revised VSAM base, while red indicates an increase. The reduction of trips in the VMC area is due to the reduction of employment within the VMC core area. This reduction in employment also resulted in the decrease of inbound traffic along Highway 407 and Highway 400. The increases in traffic outside the VMC are attributed to the additional employment along the Highway 50 and Highway 427 corridors.

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Figure 7 - Land Use Impact on Auto Network


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## N etwork Impadts

Similarly, the changes in traffic volumes shown in Figure 8 correlate closely with the changes made in the auto network (refer to Figure 3 and Table 3). The reduction of traffic in Peel Region was due to the removal of the NS roadway between Castlemore Road and Coleraine D rive (refer to Figure 4 and Table 4, item 10 ). An additional lane, along with increased capacity, was added to Zenway Boulevard in the revised VSAM 2031 base, and reflects the increased traffic volume (shown in red), as well as traffic diversion from Highway 7. The traffic reduction (shown in green) on Jane St correlates with the lane/ capacity reduction, and the increases in traffic on the smaller collector roads are sensible since they were not included in the Nov 2009 VSAM and were included in this VSAM revision.

Figure 8 - Network Impacts on Auto Network


Transit Ridership

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Transit service assumptions for headway and speed exert a direct and significant influence on transit ridership. Figure 9 compares the transit ridership differences between Scenario 50613 and Scenario 40113 (same land use, original versus revised network). G reen indicates a decrease in ridership on the new network with the new landuse, and red indicates an increase.

Figure 9-Transit Network Impacts on Transit Demand


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## Network Impads

Within the City of Vaughan, the relative increase and decrease in ridership can all be reasonably accounted for based on the transit network adjustments documented in Figure 5 and Table 5 (i.e. headway, speeds, alignment, stations locations and related access links).

## Base PM Model Results

The model results package for the revised VSAM 2031 PM base (Scenario 55813) including auto volume, auto and truck volume, v/ c ratio (with and without trucks), and various network attributes can be found in Appendix B.

A uto V olumes
In order to assess the effects of land use on auto volumes separate from auto and transit network changes and parking costs. Comparisons were made between scenarios with alternate network and land use:

|  | Orig. LU | New LU |
| :---: | :---: | :---: |
| Orig. Network | Sc45013 | Sc45113 |
| New Network | Sc55713 | Sc55813 |

In order to isolate the effects of land use changes, Scenario 55813 (new land use, new auto and transit network) was compared against Scenario 55713 (original land use, new auto and transit network). To evaluate the impacts of network changes, Scenario 55813 (new land use, new auto and transit network) was compared against 45113 (new land use, original auto and transit network).

L and U se Impacts
Overall, the differences in auto volumes within the City of Vaughan shown in Figure 10 correlate with the changes made in land use. The reduction of trips in the VMC area is due to the reduction of employment within the VMC core area. This reduction in employment also resulted in the decrease of outbound traffic along Highway 407 and Highway 400.

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Figure 10 - Land Use Impact on PM Auto Network


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## N etwork Impads

Similarly, the changes in traffic volumes shown in Figure 11 correlates closely with the changes made in the auto network. The reduction of traffic in Peel Region (north of Castlemore Rd. and West of Highway 50) was due to the removal of the NS roadway between Castlemore Road and Coleraine D rive (refer to Figure 4 and Table 4). An additional lane, along with increased capacity, was added to Zenway Boulevard in the revised VSAM 2031 base, and reflects the increased traffic volume, as well as traffic diversion from Highway 7. The traffic reduction on Jane St correlates with the lane/ capacity reduction, and the increases in traffic on the smaller collector roads are sensible since they were not included in the Nov 2009 VSAM and were included in this VSAM revision.

Figure 11- N etwork Impacts on PM Auto Network


## Alternative 1

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The 2031 Alternative 1 network was developed based on the 2031 Revised VSAM base with variations to the auto network. Some significant changes include the addition of Langstaff and Colossus bridges and the replacement of the VMC ring road network with a grid network. Table 6 and the accompanying Figure 12 provide details regarding the nature and the location of these updates.

Table 6 - Alt. 1 Auto network updates/ refinements

| ref | Revision | New Base |  | New Alt 1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Lane | Cap | Lane | Cap |
| 1 | Add NS link | na | na | 1/1 | 700 |
| 2 | Add EW link | na | na | 1/1 | 700 |
| 3 | Lane/Capacity Change | 1/1 | 400 | 2/2 | 600 |
| 4 | Add NS link | na | na | 1/1 | 400 |
| 5 | Add EW link | na | na | 1/1 | 400 |
| 6 | Add NS link | na | na | 1/1 | 400 |
| 7 | Add EW link | na | na | 1/1 | 400 |
| 8 | Add EW link | na | na | 1/1 | 400 |
| 9 | Interchange Improvement | na | na | 2/2 | 1400 |
| 10 | Add EW link | na | na | 2/2 | 600 |
| 11 | Delete links | 2/2 | 900 | na | na |
| 12 | Lane/Capacity Change | 1/1 | 400 | 2/2 | 600 |
| 13 | Add NS link | na | na | 2/2 | 600 |
| 14 | Add EW link | na | na | 2/2 | 900 |
| 15 | Add EW link | na | na | 2/2 | 600 |
| 16 | Add EW link | na | na | 2/2 | 900 |
| 17 | Add EW link | na | na | 1/1 | 400 |
| 18 | Add NS link | na | na | 1/1 | 400 |
| 19 | Add EW link | na | na | 1/1 | 400 |
| 20 | Add EW link | na | na | 1/1 | 400 |
| 21 | Add NS link | na | na | 1/1 | 400 |
| 22 | Add EW link | na | na | 1/1 | 400 |
| 23 | Add EW link | na | na | 1/1 | 900 |

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Figure 12- Altemative 1 Auto N etwork Changes


## Altemative 1AM Model Results

The model results package for the 2031 Alternative 1 AM (Scenario 70813) including auto volume, auto and truck volume, v/ c ratio (with and without trucks), transit ridership, and various network attributes can be found in Appendix C.

Network Impacts
To evaluate the impacts of network changes, Scenario 70813 was compared against 50813 (same land use and transit assumptions, different auto network). The changes in auto volume shown in Figure 12 correlate closely with the changes made in the auto network shown in Figure 12 and Table 6. The increase of traffic (shown in red) along Huntington Rd., King-Vaughan Rd. and Kirby Rd. can be attributed to the extension of Huntington Rd. between Kirby Rd. and King-Vaughan Rd. and the

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extension of Kirby Rd. between Huntington and Hwy 27 (labelled as \#1 and \#2 in Figure 12, respectively). These extensions divert some traffic away from Hwy 27 and Teston Rd (shown in green). Similarly, the providing connectivity between Dufferin St. and Bathurst St. (labelled as \#23 in Figure 12) diverted traffic away from Elgin Mills Rd. W and King-Vaughan Rd.. The addition of the Langstaff Bridge across the CN stockyards as well as the improvements at the Langstaff and Hwy 400 interchange led to significant traffic increase along Langstaff Rd. and some diversion from Rutherford Rd. Within the VMC area, the replacement of the ring road system with a grid system caused significant changes in traffic patterns. The new Colossus Bridge alleviated Highway 7 of some traffic, while the improved connections provided by the extension of Millway St. and Portage Rd. resulted in increased traffic.

Figure 13-N etwork Impacts on AM Auto N etwork


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## Altemative 1PM Model Results

The model results package for Alternative 1 PM (Scenario 75813) including auto volume, auto and truck volume, v/ c ratio (with and without trucks), and various network attributes can be found in Appendix D.

Network Impacts
In order to evaluate the impacts of network changes, Scenario 75813 was compared against 55813 (same land use and transit assumptions, different auto network). The changes in auto volume shown in Figure $\mathbf{1 4}$ are very similar to the changes shown in Figure $\mathbf{1 3}$ and can be attributed to the changes made in the auto network. An increase in traffic is shown in red and green indicates a decrease.

Figure 14 - Network Impacts on PM Alt. 1 Auto Network


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## VSAM 2021-Base

As per discussions with AECOM, a 2021 VSAM base network was developed based on 2031 VSAM Alternative 1, with minor modifications to the road network. These include the removal of the Langstaff Bridge and the Colossus Bridge, the removal of the Kirby Rd. link between Huntington Rd. and Hwy 27 and between Dufferin St. and Bathurst St., Huntington between Kirby Rd. and KingVaughan Rd., as well as the removal of the N-S local road between Major Mackenzie Dr. and Teston Rd. In addition, sections of several arterials will not be widened in 2021 and their lane assumptions also changed. Table 7 and the accompanying Figure $\mathbf{1 5}$ provide details regarding the nature and the location of these network differences.

Table 7 - 2021 Base Auto Network Modifications

| ref | Revision | 2021 Base |  | 2031 Alt 1 |  |
| :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Lane | Cap | Lane | Cap |
| 1 | Remove link | na | na | $1 / 1$ | 700 |
| 2 | Remove link | na | na | $1 / 1$ | 700 |
| 3 | Remove link | na | na | $1 / 1$ | 400 |
| 4 | Remove lane | $2 / 2$ | 900 | $3 / 3$ | 800 |
| 5 | Remove lane | $2 / 2$ | 900 | $3 / 3$ | 800 |
| 6 | Remove lane | $2 / 2$ | 800 | $3 / 3$ | 711 |
| 7 | Remove link | na | na | $2 / 2$ | 600 |
| 8 | Remove link | na | na | $2 / 2$ | 900 |
| 9 | Remove link | na | na | $1 / 1$ | 900 |

Figure 15-2021 Network vs. 2031 Network


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## L and Use

Population and employment figures within the City of Vaughan were revised by Halcrow to match the land use control totals for Vaughan provided by York Region on December 11, 2009 using interpolation between 2006 and 2031.

Figure 16 and Figure 17 show the location and magnitude of the population and employment for Year 2021, respectively. The resultant population and employment totals can be seen in Table 8 below.

Figure 16 - 2021 Population Assumptions


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Figure 17-2021 Employment Assumptions


Table 8-2021 Population and Employment City of Vaughan Totals | Date Vetted | 2021 Population Totals | 2021 Employment Totals |
| :--- | :---: | :---: |
| January 26, 2009 | 360,281 | $248,665^{*}$ |

*Adjustments were made for employment in Wonderland to reflect average fall weekday traffic (TZ 1059)

## 2021 Model Results

The model results package for the 2021 Base AM (Scenario 30813) and PM (Scenario 35813) including auto volume, auto and truck volume, $\mathrm{v} / \mathrm{c}$ ratio (with and without trucks), transit ridership, and various network attributes can be found in Appendix E and Appendix F, respectively.

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## Conclusions

The 2031 VSAM base update was carried out at the request of the City of Vaughan. HCI has incorporated revised road network lane and capacity assumptions, parking cost assumptions, transit headway, speed, and stop/station location assumptions, as well as population and employment forecasts.

As documented above, the changes in road and transit demand in the revised base scenario resulting from the changes to transportation inputs (road, transit, and parking assumptions) are both modest and logical. The changes in demand associated with the land use modifications, primarily due to substantial reductions in VMC employment and increases to West Vaughan Secondary Employment Centre employment, are consistent with the scale of the changes assumed. The major impacts are concentrated in the vicinity of the VMC, particularly on Avenue 7 in the vicinity of Highway 400 where PM peak hour demands were reduced by approximately 700 vehicles.

The transit ridership was reduced along Major Mackenzie Dr., in line with the assumed changes to speed and headway which reflect the planned 2031 transit services in the YTMP Update and experience in comparable urban corridors. Peak hour ridership estimates decreased substantially as a result of reduced speeds and lower headways for both 2021 and 2031. Whereas the earlier peak hour demand estimates justified frequencies of 7 to 8 buses per hour on Major Mackenzie Dr. west of Weston Road, the assumed transit service changes resulted in AM peak hour demand levels of less than 227 in 2031.

The 2031 Alternative 1 scenario was developed based on the 2031 revised base network, with changes to the auto network. These changes redistribute traffic along parallel links that were extended, added or widened, especially within the VMC area.

The 2021 VSAM base was developed based on 2031 Alternative 1, with minor modifications to the road network. There are no differences in the 2021 transit network input assumptions when compared to the 2031 transit network. The resulting transit ridership was also similar to 2031 such that it achieved AM peak hour demand levels of less than 218 on Major Mackenzie Dr west of Weston. With the 2021 land use control totals being marginally less than the 2031 land use control totals, the overall 2021 results are very similar to the 2031 results. The $\mathrm{V} / \mathrm{C}$ ratio plots indicate that congestion occurs on most major facilities by 2021, and widening improvements are necessary in order to accommodate 2031 forecasts. The VMC area and Avenue 7 are operating at an acceptable level of service, but Avenue 7 is assumed to be six lanes by 2021 in this analysis.

Appendix A
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Revised VSAM 2031 Base AM

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Appendix B
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## Revised VSAM 2031 Base PM







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Appendix C
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Revised VSAM 2031 Alt AM

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Appendix D
Sc 75813
Revised VSAM 2031 Alt PM
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Appendix E
Sc 30813
Revised VSAM 2021 AM



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## Appendix F <br> Sc 35813 <br> Revised VSAM 2021 PM


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F2. Model Applications (Halcrow)
F2.1 2021 Base Network - AM Peak Hour
F2.2 2021 Base Network - PM Peak Hour
F2.3 2031 Base Network - AM Peak Hour
F2.4 2031 Base Network - PM Peak Hour
F2.5 2031 Alternative Network - AM Peak Hour
F2.6 2031 Alternative Network - PM Peak Hour

## F2.1 2021 Base Network - AM Peak Hour






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 2021 AM Number of Lanes 1 TOT



## F2.2 2021 Base Network - PM Peak Hour









F2.3 2031 Base Network - AM Peak Hour





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 2031 AM Number of Lanes 1 IT I



## F2.4 2031 Base Network - PM Peak Hour









F2.5 2031 Alternative Network - AM Peak Hour





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F2.6 2031 Alternative Network - PM Peak Hour
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F3. 2021 and 2031 Corridor Deficiencies

## Appendix F3

## 2021 and 2031 Corridor Deficiencies

## 1. Deficiency Analysis Methodology

### 1.1 Screenlines

Screenlines are imaginary lines located in a grid pattern over a transportation network to evaluate the completeness and accuracy of predicted trips in calibrating travel demand models by comparing simulated trip volumes on the facilities with those actually observed. Also screenline counts are used to conduct deficiency analysis with the aim of assessing the existing or planned road network capacity within a study area. According to Meyer and Miller (2001) because the validity of a screenline analysis depends on considering as many of the roads that cross the screenline as possible, the screenlines should be chosen very carefully. Often, barriers to traffic flow (e.g., rivers or railroad tracks) are used because they "funnel" movements through a small number of crossings, making total movements easier to count.

For the purpose of this study, the screenlines were defined as follows.

## East-West Screenlines extending from Highway 50 to Bathurst Street:

1. North of Steeles Avenue;
2. North of Highway 7;
3. North of Rutherford Road;
4. North of Teston Road; and
5. North of King-Vaughan Road

## North-South Screenlines extending from Steeles Avenue to King-Vaughan Road:

1. East of Highway 50;
2. East of Highway 27;
3. East of Pine Valley Drive;
4. East of Highway 400;
5. East of Keele Street; and
6. West of Bathurst Street

### 1.2 Corridors

The transportation facilities (roads, transit services, and bike paths / lanes) crossing the screenlines can be grouped into larger analysis units reflecting corridors of travel in both east-west and northsouth directions of travel. Traffic data collected at the individual facility level can be aggregated to
these larger geographical units for analysis undertaken at this level and for calibration / validation of travel forecasting models. Corridor level analysis is an accepted methodological approach in transportation planning. Master Plan and area-wide transportation studies focus on well-defined travel corridors for which very specific strategies can be considered at a relatively fine level of detail.

For the purpose of this study, the study team defined the following travel corridors:

## East-West (E-W) Corridors

1. Kirby Corridor including all the east-west roads from Teston Road (including Teston Road) to King-Vaughan Road (including King-Vaughan Road);
2. Major Mackenzie - Rutherford (Major Mac - Rutherford) Corridor including all the east-west roads from north of Langstaff Road (excluding Langstaff Road) to south of Teston Road (excluding Teston Road); and
3. Highway 407 Corridor including all the east-west roads from Steeles Avenue (including Steeles Avenue) to Langstaff Road (including Langstaff Road)

## North-South (N-S) Corridors

1. Highway 427 Corridor including all the north-south roads from Highway 50 (including Highway 50) to Highway 27 (including Highway 27);
2. Islington Corridor including all the north-south roads from Highway 27 (excluding Highway 27) to Weston Road (excluding Weston Road);
3. Highway 400 Corridor including all the north-south roads from Weston Road (including Weston Road) to Keele Street (excluding Keele Street); and
4. Dufferin Corridor including all the north-south roads from Keele Street (including Keele Street) to Bathurst Street (including Bathurst Street)

### 1.3 Summary of Expected Growth by Corridor

The following sections deal with the projections for growth in population and employment in the City of Vaughan between 2006 and 2031.

### 1.3.1 Population

As can be seen in Table 1, between 2006 and 2031, the population of the City of Vaughan is estimated to grow by $67 \%$ (from 253,000 to over 423,000). Also Table 1 illustrates that the population growth rate is predicted to slow significantly in all the corridors as of 2021 (in comparison, the average annual rate of population increase in the City of Vaughan between 2006 and 2021 is predicted to be 2 percent, whereas between 2021 and 2031 it is predicted to be only 1 percent). Overall 66\% percent of the population growth projected between 2006 and 2031 is expected to occur by 2021.

As can be seen in Figure 1, among E-W corridors, the Major Mac - Rutherford and Highway 407 corridors are predicted to accommodate $95 \%$ of overall projected population growth in the City of Vaughan between 2006 and 2031 with growth shares of $55 \%$ and $40 \%$ respectively. Although according to Figure 1, the population growth share is the lowest for the Kirby corridor (5\%), Table 1 illustrates that this corridor is predicted to have the largest annual growth rate among the E-W corridors (4\% between 2006 and 2021; 2\% between 2021 and 2031). Also according to Table 1, the Highway 407 corridor is expected to have the smallest annual growth rate ( $2 \%$ between 2006 and 2021; 1\% between 2021 and 2031).

Table 1. Population and Employment Growth by Corridor (2006-2031)

| Corridors | Data Item | Population |  |  | Employment |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 2006 | 2021 | 2031 | 2006 | 2021 | 2031 |
| Highway 427 Corridor | Absolute Number | 1422 | 12755 | 19125 | 6557 | 45891 | 57315 |
|  | Absolute Growth * |  | 11333 | 6370 |  | 39334 | 11424 |
|  | Annual Growth Rate |  | 16\% | 4\% |  | 14\% | 2\% |
|  | Relative Growth \% |  | 64\% | 36\% |  | 77\% | 23\% |
| Islington Corridor | Absolute Number | 95998 | 115256 | 123987 | 33630 | 37202 | 37202 |
|  | Absolute Growth |  | 19258 | 8731 |  | 3572 | 0 |
|  | Annual Growth Rate |  | 1\% | 1\% |  | 1\% | 0\% |
|  | Relative Growth \% |  | 69\% | 31\% |  | 100\% | 0\% |
| Highway 400 Corridor | Absolute Number | 49831 | 89829 | 111135 | 64183 | 102329 | 104821 |
|  | Absolute Growth |  | 39998 | 21306 |  | 38146 | 2492 |
|  | Annual Growth Rate |  | 4\% | 2\% |  | 3\% | 0\% |
|  | Relative Growth \% |  | 65\% | 35\% |  | 94\% | 6\% |
| Dufferin Corridor | Absolute Number | 105640 | 147335 | 168960 | 55494 | 64566 | 64566 |
|  | Absolute Growth |  | 41695 | 21625 |  | 9072 | 0 |
|  | Annual Growth Rate |  | 2\% | 1\% |  | 1\% | 0\% |
|  | Relative Growth \% |  | 66\% | 34\% |  | 100\% | 0\% |
| Kirby Corridor | Absolute Number | 7556 | 13315 | 16175 | 1837 | 21396 | 23888 |
|  | Absolute Growth |  | 5759 | 2860 |  | 19559 | 2492 |
|  | Annual Growth Rate |  | 4\% | 2\% |  | 18\% | 1\% |
|  | Relative Growth \% |  | 67\% | 33\% |  | 89\% | 11\% |
| Major Mac Rutherford Corridor | Absolute Number | 130538 | 191389 | 224670 | 51590 | 86835 | 92634 |
|  | Absolute Growth |  | 60851 | 33281 |  | 35245 | 5799 |
|  | Annual Growth Rate |  | 3\% | 2\% |  | 4\% | 1\% |
|  | Relative Growth \% |  | 65\% | 35\% |  | 86\% | 14\% |
| Highway 407 Corridor | Absolute Number | 114797 | 160471 | 182362 | 106437 | 141757 | 147382 |
|  | Absolute Growth |  | 45674 | 21891 |  | 35320 | 5625 |
|  | Annual Growth Rate |  | 2\% | 1\% |  | 2\% | 0\% |
|  | Relative Growth \% |  | 68\% | 32\% |  | 86\% | 14\% |
| Total City of Vaughan | Absolute Number | 252891 | 365175 | 423207 | 159864 | 249988 | 263904 |
|  | Absolute Growth |  | 112284 | 58032 |  | 90124 | 13916 |
|  | Annual Growth Rate |  | 2\% | 1\% |  | 3\% | 1\% |
|  | Relative Growth \% |  | 66\% | 34\% |  | 87\% | 13\% |

Note: * The first number is the 15 year growth between 2006 and 2021, and the second one is the 10 year growth between 2021 and 2031

Figure 1 also shows the projected population growth shares for N-S corridors with the Dufferin and Highway 400 corridors predicted to almost equally accommodate $73 \%$ of the overall population growth between 2006 and 2031. Again as can be seen from Figure 1 and Table 1, although the Highway 427 corridor is expected to have the lowest population growth share (11\%) among the N-S corridors, it is
predicted to show the largest annual growth rate (16\%) between 2006 and 2021 by a significant margin in comparison to those growth rates in other E-W corridors. Also as can be seen in Table 1, the Islington corridor is estimated to have the lowest annual growth rate (1\% between 2006 and 2031).


Figure 1. Projected Population Growth Share for Defined Corridors between 2006 and 2031

### 1.3.2 Employment

Table 1 illustrates that the number of workers in the labour force between 2006 and 2031 is predicted to increase from 160,000 to 264,000, at almost the same rate that the population grows. Also Table 1 illustrates that virtually all of the employment growth predicted to occur by 2031 ( $87 \%$ ) is expected by 2021 (in comparison, the average annual rate of employment increase in the City of Vaughan between 2006 and 2021 is predicted to be 3 percent, whereas between 2021 and 2031 it is predicted to be only 1 percent). Also it is essential to note that the employment growth in E-W corridors is estimated to occur for the most part ( $86 \%$ to $89 \%$ ) between 2006 and 2021, following the same overall trend as the City as a whole. However except for the Highway 427 corridor, all of the expected employment growth in N-S corridors is predicted to be achieved by 2021.


Figure 2. Projected Employment Growth Share for Defined Corridors between 2006 and 2031

As can be seen in Figure 2, among E-W corridors, $79 \%$ of overall projected employment growth in the City of Vaughan between 2006 and 2031 is predicted to occur within the Major Mackenzie Rutherford and Highway 407 corridors with almost equal growth shares of $40 \%$ and $39 \%$ respectively. Although according to Figure 2, the employment growth share is the lowest for the Kirby corridor ( $21 \%$ ), the Table 1 illustrates that this corridor is predicted to have the largest annual employment growth rate among the E-W corridors by a significant margin (18\% between 2006 and 2021; 1\% between 2021 and 2031). Also according to Table 1, the Highway 407 corridor is expected to have the smallest annual employment growth rate ( $2 \%$ between 2006 and 2021; 0\% between 2021 and 2031).

Figure 2 shows the employment growth shares for N-S corridors with the Highway 427 corridor predicted to accommodate almost half of the overall growth in employment between 2006 and 2031, followed by the Highway 400 corridor (estimated to accommodate $39 \%$ of overall employment growth in the City). These two corridors together are predicted to have an $88 \%$ share of the overall employment growth between 2006 and 2031. Also as can be seen in Figure 2, the Islington corridor is expected to have the lowest share (3\%) of overall employment growth in the City between 2006 and 2031. As can be seen from Table 1, the Highway 427 corridor is expected to have the largest annual growth rate (14\% between 2006 and 2021; 2\% between 2021 and 2031) by a significant margin in comparison to those growth rates in other N-S corridors, whereas the Islington and Dufferin corridors are estimated to have the lowest annual growth rate (1\% between 2006 and 2031).

### 1.4 Road Network Improvement Assumptions

The future growth in population and employment will result in increased travel in the City of Vaughan and it will place considerable strain on its road system unless a long-term transportation vision, and integrated road and transit plan supportive of the growth is defined. For many years, managing the transportation system by adding new lanes or new roads or by making operational changes to improve the system performance has been the most common response to address future travel needs and to avoid possible transportation problems. Table 2 shows the entire assumed road network improvements considered in the network testing for this study. These improvements were evaluated as potential solutions with the aim of selecting the preferred improvements to keep pace with the travel needs of a growing population and expanding employment base in the City of Vaughan. The typical road network improvements include building new roads, new interchanges, railway grade separations with intersecting roads, and road widenings (see Table 2). Some of these improvements are major initiatives that have already been planned to be constructed as part of Provincial and Regional plans, as noted below:

ا The Highway 427 extension northerly from Highway 7 to Major Mackenzie Drive (with 3 new interchanges);

- Highway 400 widening north of Major Mackenzie Drive;
> Interchange improvements on Highway 400 at Langstaff Road, Highway 7, and Steeles Avenue and on Highway 407 at Centre Street;

| Table 2. Assumed Road Improvements in the City of Vaughan |  |  |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| Direction | Corridor | Provincial Roads |  | Regional Roads |  | Local Roads |  |
|  |  | Road Improvement | Limits/Location | Road Improvement | Limits/Location | Road Improvement | Limits/Location |
|  | Highway 407 Corridor | - New Interchange <br> - Interchange Improvements |  | - Langstaff Road Widening (2 to 4) <br> - Langstaff Extension (New 4-lane) <br> - Langstaff Road Widening (2 to 4) <br> - Steeles Avenue Widening (4 to 6) <br> - Steeles Avenue Widening (4 to 6 ) <br> - Highway 7 Widening (4 to 6) |  | - Portage Parkway <br> Extension (2-Lane) <br> - New E-W 4-Lane Road <br> - Colossus Drive <br> Extension (4-Lane) <br> - Interchange Way <br> Extension (2 Lane) <br> - New Highway 400 NB On Ramp (Link 5) | - Jane Street to Creditstone Road <br> - North of Steeles between Jane Street and Keele Street <br> - Colossus Drive to Interchange Way <br> - Jane Street to Creditstone Road <br> - Link 4 to Highway 400 NB |
|  | Major Mackenzie Corridor |  |  | - Rutherford Road <br> Widening (4 to 6) <br> - Major Mackenzie Drive <br> Widening (2 to 6) <br> - Major Mackenzie Drive Widening (4 to 6) | - Highway 50 to Weston Road <br> - Highway 50 to Weston Road <br> - Weston Road to Bathurst Street | - New Highway 400 Crossing (4-Lane) <br> - New Highway 400 Crossing (4-Lane) <br> - New E-W 4-Lane Local Road <br> - New E-W 2-Lane Local Road <br> - New E-W 2-Lane Local Road <br> - New E-W 4-Lane Local Road | - North of Major Mackenzie Drive (North Maple Community Bridge) <br> - North of Rutherford Road <br> - South of Rutherford Road between Highway 50 and Highway 27 <br> - South of Rutherford Road, West of Weston Road to Highway 400 <br> - South of Rutherford Road, East of Highway 400 <br> - North of Major Mackenzie Drive between Highway 50 and Huntington Road |
|  | Kirby Corridor |  |  | - Teston Road Widening (2 to 4) <br> - Teston Road (New 4 Lane) <br> - Teston Road Widening (2 to 4) <br> - King-Vaughan Road Widening (2 to 4) <br> - Kirby Road Widening (2 to 4) <br> - Kirby Road (New 4-Lane) | Pine Valley Drive to Highway 400 Keele Street to Dufferin Street $>$ Bathurst Street to Dufferin Street $>$ Bathurst Street to Pine Valley Drive $>$ Keele Street to Dufferin Street $>$ Dufferin Street to Bathurst Street | - Highway 400 Crossing | - North of Kirby Road (Subject to Results of GTA West Studies) |

Table 2. Assumed Road Improvements in the City of Vaughan

| Direction | Corridor | Provincial Roads |  | Regional Roads |  | Local Roads |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | Road Improvement | Limits/Location | Road Improvement | Limits/Location | Road Improvement | Limits/Location |
|  | Highway 427 Corridor | Highway 427 Extension <br> Interchange Improvement <br> - New Interchange <br> - New Interchange <br> - New Interchange | Highway 7 to Major Mackenzie Drive <br> Highway 7 / <br> Highway 427 <br> Langstaff Road / <br> Highway 427 <br> Rutherford Road / <br> Highway 427 <br> Major Mackenzie Drive / <br> Highway 427 | Highway 50 Widening (4 to 6) <br> Highway 27 Widening (4 to 6) Highway 27 Widening (2 to 4) | Steeles Avenue to the South of Kirby Road Steeles Avenue to Major Mackenzie Drive Major Mackenzie Drive to King-Vaughan Road | - Huntington Road Widening (2 to 4) New N-S 2-Lane Local Road 2 New Local N-S 2Lane Roads <br> New Huntington Road <br> 2 New N-S 2-Lane Local Roads <br> MacGillvray Road | Langstaff Road <br> Drive to Kirby Road <br> North of Highway 7 to Zenway Boulevard <br> Langstaff Road to Rutherford Road (west of Huntington Road) <br> Highway 7 to Langstaff Road <br> Major Mackenzie to Nashville Road (east and west of Huntington Rd) Langstaff Road to south of Major Mackenzie Dr |
|  | Islington Corridor |  |  | Pine Valley Drive Widening (4 to 6) | Steeles Avenue to Highway 7 | - |  |
|  | Highway 400 Corridor | Highway 400 Widening (6 to 10) Interchange Improvements Interchange Improvements Interchange Improvement New Interchange | Major Mackenzie Drive to King-Vaughan Road Langstaff / Highway 400 Steeles / Highway 400 Highway 7 / Highway 400 King-Vaughan / Highway 400 | Weston Road Widening (2 to 4) <br> Weston Road Widening (4 to 6) Jane Street Widening (2 to 4) Jane Street Widening (4 to 6) | Major Mackenzie Drive to Kirby Road Steeles Avenue to Major Mackenzie Drive Teston Road to Kirby Road Steeles Avenue to Major Mackenzie Drive | Millway Avenue Extension (4 Lanes) Local Roads (New) Millway Avenue Widening (2 to 4) Creditstone Road Widening (2 to 4) New Extension of Highway 400 NB off Ramp (Link 4) 2 New N-S 2-Lane Local Roads New N-S 2-Lane Local Road | South of Highway 7 to Interchange Way East and West of Weston Road North of Highway 7 to Portage Parkway <br> Peeler Road to Langstaff Road <br> Highway 7 to Portage Parkway <br> West of Weston Road, North of Major Mackenzie Drive East of Highway 400 from Teston Road to King-Vaughan Road |
|  | Dufferin Corridor |  |  | Keele Street Widening (4 to 6) <br> Bathurst Street Widening (4 to 6) <br> Dufferin Street Widening (2 to 4) <br> Dufferin Street Widening (4 to 6) | Steeles Avenue to Major Mackenzie Drive South of Rutherford Road to Teston Road Major Mackenzie Drive to King-Vaughan Road Langstaff Road to Major Mackenzie Drive |  |  |

* A new interchange on Highway 407 at Martin Grove Road;

〉 Widenings of Major Mackenzie Drive, Rutherford Road, Highway 27, Weston Road, and Pine Valley Drive that are being recommended by the Region of York through the Western Vaughan Transportation Improvements Individual Environmental Assessment (IEA);
> Other widenings of Regional roads east of Highway 400, including sections of Jane Street, Keele Street, Dufferin Street and Bathurst Street in the north-south direction; and widenings of Major Mackenzie Drive and sections of Langstaff Road and Teston Road, together with construction of the Langstaff and Teston missing links; and
) New and improved railway grade separations associated with the above-noted Western Vaughan IEA recommended arterial widenings.

### 1.5 Corridor Deficiencies

Figures 3 to 10 show the volume-to-capacity ratios by direction along all of the $\mathrm{N}-\mathrm{S}$ and $\mathrm{E}-\mathrm{W}$ corridors during both the AM and PM peak hours in 2021 and 2031. These figures identify the corridor deficiencies and provide the basis for more detailed corridor analysis of needed improvements for 2021 documented in Appendix L.







Figure 8. Level of Senvice for N-S Corridors in the City of Vaughan during AM Peak Period in 2021




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    Halcrow Consulting Inc has prepared this report in accordance with the instructions of their client, City of Vaughan, for their sole and specific use. Any other persons who use any information contained herein do so at their own risk.

[^1]:    ${ }^{1}$ York Aerial Photographs. York Region. Available at [http://maps.york.ca/imf/imf.jsp?site=geoRegOrtho](http://maps.york.ca/imf/imf.jsp?site=geoRegOrtho)

[^2]:    2.5 .2

[^3]:    2031 Population Differences

